

APPENDIX M

YEAR 2030 WITH PROJECT BUILD-OUT SYNCHRO WORKSHEETS

HCM Signalized Intersection Capacity Analysis
1: Via De La Valle & El Camino Real

Year 2030 + Project AM
12/1/2010
HCM Signalized Intersection Capacity Analysis
2: San Dieguito Road & El Camino Real

Movement	→	→	←	←	↑	↑	↓	↓
Lane Configurations								
Lane Volume (vph)	10	521	701	460	10	160	5	202
Total Volume (vph)	1910	1910	1910	1910	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	1.00	1.00	1.00
Filt. Permitted	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Filt. Rejected	0.95	1.00	1.00	0.95	0.95	1.00	1.00	0.98
Stat. Flow (proj.)	17.0	55.9	56.3	34.3	35.8	16.1	16.7	16.3
Filt. Permitted	0.95	1.00	1.00	0.95	0.95	1.00	1.00	0.99
Stat. Flow (perm.)	17.0	55.9	56.3	34.3	35.8	16.1	16.7	16.3
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	11	32.2	57.9	37.9	51.1	31.1	31.1	31.1
RTO/R Reduction (vph)	0	0	408	0	2	0	0	0
Lane Group Flow (vph)	11	422	717	779	7520	0	261	256
Turn Type	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm
Protected Phases	1	2	1	2	1	2	1	2
Permitted Phases	1	2	1	2	1	2	1	2
Achieved Green (G, s)	0.6	1.7	1.7	1.4	1.4	1.6	1.6	1.7
Effective Green (g, s)	0.6	1.7	1.7	1.4	1.4	1.6	1.6	1.7
Achieved g/C Ratio	0.01	0.30	0.30	0.25	0.25	0.35	0.35	0.35
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	24	1045	460	851	1869	328	316	400
Vs Ratio (vph)	0.01	0.12	0.23	0.15	0.15	0.16	0.16	0.16
Vs Ratio Perm	0.11	0.20	0.04	0.02	0.02	0.02	0.02	0.02
Vs Ratio	0.16	0.40	0.37	0.92	0.26	0.79	0.81	0.81
Uniform Delay, d1	20.8	16.6	16.4	21.5	7.6	20.6	20.7	16.7
Progression Factor, F1	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Incremental Delay, d2	13.2	0.3	0.5	11.3	0.1	12.3	14.5	0.2
Delay (s)	21	16.9	16.3	35.6	7.7	33.1	35.1	16.8
Level of Service	O	B	B	D	A	C	B	B
Approach LOS	B	B	B	C	C	C	C	B

Intersection Summary	C
HCM Average Control Delay	26.7
HCM Volume to Capacity ratio	0.66
Actuated Cycle Length (s)	12.0
Intersection Capacity Utilization	B
Analysis Period (min)	15
Critical Lane Group	C

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
3. Derby Downs Road & El Camino Real

Year 2030 + Project AM
12/1/2010

HCM Signalized Intersection Capacity Analysis
4. Half Mile Road & El Camino Real

Year 2030 + Project AM
12/1/2010

Movement	1	2	3	4	5	6	7	8	9	10	11
Lane Configurations											
Volume (vph)	0	516	53	114	114	160	160	165	174	174	174
Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vph)	1910	1910	1910	1910	1910	1910	1910	1910	1910	1910	1910
Total Loss (Time (s))	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flow	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Flt Permitted	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Flt Projected	0.96	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Solid Flow (vph)	3414	3411	3411	3411	3411	3411	3411	3411	3411	3411	3411
Flt Permitted	0.96	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Solid Flow (perm)	3411	3411	3411	3411	3411	3411	3411	3411	3411	3411	3411
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	106	119	110	106	106	117	117	117	117	117	117
ROTOR Reduction (vph)	8	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	107	107	107	107	107	118	118	118	118	118	118
Tun Type	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.
Projected Phases	5	5	5	5	5	5	5	5	5	5	5
Permitted Phases	5	5	5	5	5	5	5	5	5	5	5
Actuated Green, G (s)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Effective Green, g (s)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Actuated QC Ratio	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10
Clearance Time, t _c (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap. Cap. (vph)	341	341	341	341	341	342	342	342	342	342	342
Vs Ratio Proj.	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
Vs Ratio Perm	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
Yc Ratio	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31
Uniform Delay, d ₁	14.1	14.1	14.1	14.1	14.1	16.5	16.5	16.5	16.5	16.5	16.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d ₂	181.3	181.3	181.3	181.3	181.3	186.5	186.5	186.5	186.5	186.5	186.5
Delay (s)	205.9	205.9	205.9	205.9	205.9	216.7	216.7	216.7	216.7	216.7	216.7
Level of Service	F	F	F	F	F	E	E	E	E	E	E
Approach Delay (s)	47.7	47.7	47.7	47.7	47.7	55.5	55.5	55.5	55.5	55.5	55.5
Approach LOS	B	B	B	B	B	C	C	C	C	C	C
Intersection Summary											
HCM Average Control Delay	4.3	HCM Level of Service	A								
HCM Volume to Capacity Ratio	0.93	Sum of lost time (s)	80								
Adjusted Cycle Length (s)	33.7	[C] Level of Service	A								
Intersection Capacity Utilization	66.7%	Analysis Period (min)	15								
Critical Lane Group		Baseline									

Intersection Summary	
HCM Average Control Delay	24.8
HCM Volume to Capacity Ratio	0.97
Adjusted Cycle Length (s)	50.0
Intersection Capacity Utilization	99.5%
Analysis Period (min)	15
Critical Lane Group	

Movement	1	2	3	4	5	6	7	8	9	10	11
Lane Configurations											
Volume (vph)	0	516	53	114	114	160	160	165	174	174	174
Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vph)	1910	1910	1910	1910	1910	1910	1910	1910	1910	1910	1910
Total Lost (Time (s))	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flow	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Flt Permitted	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Flt Projected	0.96	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Solid Flow (vph)	3414	3411	3411	3411	3411	3411	3411	3411	3411	3411	3411
Flt Permitted	0.96	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Solid Flow (perm)	3411	3411	3411	3411	3411	3411	3411	3411	3411	3411	3411
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	106	119	110	106	106	117	117	117	117	117	117
ROTOR Reduction (vph)	8	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	107	107	107	107	107	118	118	118	118	118	118
Tun Type	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.
Projected Phases	5	5	5	5	5	5	5	5	5	5	5
Permitted Phases	5	5	5	5	5	5	5	5	5	5	5
Actuated Green, G (s)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Effective Green, g (s)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Actuated QC Ratio	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10
Clearance Time, t _c (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap. Cap. (vph)	341	341	341	341	341	342	342	342	342	342	342
Vs Ratio Proj.	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
Vs Ratio Perm	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
Yc Ratio	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31
Uniform Delay, d ₁	14.1	14.1	14.1	14.1	14.1	16.5	16.5	16.5	16.5	16.5	16.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d ₂	181.3	181.3	181.3	181.3	181.3	186.5	186.5	186.5	186.5	186.5	186.5
Delay (s)	205.9	205.9	205.9	205.9	205.9	216.7	216.7	216.7	216.7	216.7	216.7
Level of Service	F	F	F	F	F	E	E	E	E	E	E
Approach Delay (s)	47.7	47.7	47.7	47.7	47.7	55.5	55.5	55.5	55.5	55.5	55.5
Approach LOS	B	B	B	B	B	C	C	C	C	C	C
HCM Average Control Delay											
HCM Volume to Capacity Ratio	0.93	HCM Level of Service	A								
Adjusted Cycle Length (s)	33.7	Sum of lost time (s)	80								
Intersection Capacity Utilization	66.7%	[C] Level of Service	A								
Analysis Period (min)	15	Critical Lane Group									

Intersection Summary	
HCM Average Control Delay	24.8
HCM Volume to Capacity Ratio	0.97
Adjusted Cycle Length (s)	50.0
Intersection Capacity Utilization	99.5%
Analysis Period (min)	15
Critical Lane Group	

Intersection Summary	
HCM Average Control Delay	24.8
HCM Volume to Capacity Ratio	0.97
Adjusted Cycle Length (s)	50.0
Intersection Capacity Utilization	99.5%
Analysis Period (min)	15
Critical Lane Group	

Intersection Summary	
HCM Average Control Delay	24.8
HCM Volume to Capacity Ratio	0.97
Adjusted Cycle Length (s)	50.0
Intersection Capacity Utilization	99.5%
Analysis Period (min)	15
Critical Lane Group	

Intersection Summary	
HCM Average Control Delay	24.8
HCM Volume to Capacity Ratio	0.97
Adjusted Cycle Length (s)	50.0
Intersection Capacity Utilization	99.5%
Analysis Period (min)	15
Critical Lane Group	

Intersection Summary	
HCM Average Control Delay	24.8
HCM Volume to Capacity Ratio	0.97
Adjusted Cycle Length (s)	50.

HCM Unsigneded Intersection Capacity Analysis
7: Del Mar Heights Road & Portofino Drive

Year 2030 + Project AM
12/1/2010
8: Del Mar Heights Rd. & I-15 SB Ramps

	Approach LOS	Approach Delay (s)	Intersection Summation	Intersection Capacity Utilization (%)	Analysis Period (min)
Lane LOS	B	0.0	0.0	0.6	15
Approach Delay (s)	B	0.0	0.0	0.6	15
Approach LOS	B	0.0	0.0	0.6	15
Volume Right (Vrh)	67	0	0	0.6	15
6SH	1100	1100	1700	925	15
Volume to Capacity	0.40	0.40	0.26	0.65	15
Ozone Length 95th (O)	0	0	0	0	24
Control Delay (s)	0.0	0.0	0.0	0.0	0.0
Volume Total	0	0	0	0	0
Volume Right	67	0	0	0	0
6SH	1100	1100	1700	925	15
Volume to Capacity	0.40	0.40	0.26	0.65	15
Ozone Length 95th (O)	0	0	0	0	24
Control Delay (s)	0.0	0.0	0.0	0.0	0.0
Lane LOS	B	0	0	0.6	15
Approach LOS	B	0.0	0.0	0.6	15
Approach Delay (s)	B	0.0	0.0	0.6	15
Intersection Summation					
Average Delay					
Intersection Capacity Utilization (%)				150.4%	
Analysis Period (min)				15	

HCM Signalized Intersection Capacity Analysis
8: Del Mar Heights Rd. & I-15 SB Ramps

Year 2030 + Project AM
4/26/2011
7: Del Mar Heights Rd. & Portofino Drive

	Approach LOS	Approach Delay (s)	Intersection Summation	Intersection Capacity Utilization (%)	Analysis Period (min)
Lane LOS	B	0.0	0.0	0.6	15
Approach LOS	B	0.0	0.0	0.6	15
Approach Delay (s)	B	0.0	0.0	0.6	15
Intersection Summation					
Average Delay					
Intersection Capacity Utilization (%)				150.4%	
Analysis Period (min)				15	

HCM Signalized Intersection Capacity Analysis
Year 2030 + Project AM
12/1/2010

HCM Signalized Intersection Capacity Analysis
12: Del Mar Heights Road & First Ave.

Year 2030 + Project AM
12/1/2010

	Approach LOS	A	B	C	A	B	C	A	B	C	A	B	C
Intersection Summary													
HCM Average Control Delay	8.3	HCM Level of Service	A										
HCM Volume to Capacity Ratio	0.65												
Actuated Cycle Length (s)	120	Sum of LOS Line (s)	120										
Intersection Capacity Utilization	58.0	HCM Level of Service	B										
Analysis Period (min)	15	Sum of LOS Line (s)	15										
c : Critical Lane Group		c : Critical Lane Group											

	Turn Type	Perm	Prot										
Permitted Phases													
Actuated Green, G(s)	39.3	39.3	39.3	39.3	39.3	39.3	39.3	39.3	39.3	39.3	39.3	39.3	39.3
Effective Green, g(s)	32.3	32.3	32.3	32.3	32.3	32.3	32.3	32.3	32.3	32.3	32.3	32.3	32.3
Actuated g/C Ratio	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	2832	802	244	3804	337	156							
Vs Ratio Prot	0.07	0.08	0.09	0.07	0.08	0.09							
Vs Ratio Perm	0.07	0.06	0.05	0.07	0.06	0.05							
Uniform Delay, dJ	7.6	5.0	22.9	3.9	20.1	19.4							
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00							
Incremental Delay, dI	0.8	0.8	0.1	11.5	0.5	1.0							
Delay (s)	6.6	6.6	5.0	5.0	5.0	5.0							
Level of Service	A	A	C	A	C	B							
Approach Delay (s)	8.3	8.3	8.3	8.3	8.3	8.3							
Approach LOS	A	A	C	A	C	B							

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HCM Signalized Intersection Capacity Analysis	
21: Carmel Creek Road & Carmel Country Road	
Lane Group Flow (vph)	316
Lane Extension (vph)	316
Lane Grip Cap (vph)	319
HS Ratio Pct.	20.16
HS Ratio Perm	0.07
HS Ratio	0.91
Uniform Delay, d1	35.0
Progression Factor	1.00
Incremental Delay, d2	25.9
Delay (s)	0.1
Level of Service	E
Approach Delay (s)	10.3
Approach LOS	D
Intersection LOS	D
HCM Average Control Delay	45.7
HCM Volume to Capacity ratio	0.91
Actuated Cycle Length (s)	16.0
Intersection Capacity Utilization	85.2%
Analysis Period (min)	5
c: Critical Lane Group	

HCM Signalized Intersection Capacity Analysis	
22: High Bluff Drive & El Camino Real	
Movement	W-E
Lane Configurations	11
Volume (vph)	264
Ideal Flow (vph)	192
Total Lost time (s)	4.0
Lane Util. Factor	1.00
Flt.	1.00
Flt Protected	0.95
Side Flow (vph)	1770
Flt Permitted	0.95
Sid. Flow (perm)	1770
Peak-hour Factor PHF	0.90
Adj. Flow (vph)	174
RTO Reduction (vph)	0
Lane Group Flow (vph)	1770
Tun Type	Perm
Protected Phases	2
Permitted Phases	2
Actuated Green, G (s)	10.0
Effective Green, g (s)	10.0
Actuated G/C Ratio	0.18
Clearance Time (s)	10
Vehicle Extension (s)	30
Lane Grip Cap (vph)	321
vs Reloc Priority	0.07
vs Felt Perm	0.08
vs Felt	0.08
Uniform Delay, d1	18.9
Progression Factor	1.00
Incremental Delay, d2	0.2
Delay (s)	0
Level of Service	B
Approach Delay (s)	10.3
Approach LOS	B
Intersection LOS	D
Intersection Summary	
HCM Average Control Delay	24.4
HCM Volume to Capacity ratio	0.70
Actuated Cycle Length (s)	55.1
Intersection Capacity Utilization	67.0%
Analysis Period (min)	15
c: Critical Lane Group	

Synchro 7 Report
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Baseline

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HCM Unsignalized Intersection Capacity Analysis		Year 2030 + Project AM 12/1/2010			
23: High Bluff Drive & Carmel Vista Road					
Intersection Summary					
Delay					
HCM Level of Service	A	Intersection Capacity Utilization	0.69		
Analysis Period (min)	15	Outflow of Service	0.69		
Approach LOS	A	Approach LOS	A		
Approach Delay (s)	8.2	Approach Delay (s)	9.0		
Lane Group Summary		Lane Group Summary			
Volume (vph)	77	Volume (vph)	97		
Volume: Left (vph)	0	Volume: Left (vph)	0		
Volume: Right (vph)	77	Volume: Right (vph)	97		
Departure Headways (s)	0.40	Departure Headways (s)	0.67		
Degree Utilization (%)	0.13	Degree Utilization (%)	0.19		
Capacity (vph/h)	534	Capacity (vph/h)	650		
Control Delay (s)	9.0	Control Delay (s)	7.7		
Approach LOS	A	Approach LOS	B		
Approach Delay (s)	8.2	Approach Delay (s)	9.0		
Intersection LOS	A	Intersection LOS	B		
Intersection Delay (s)	9.3	Intersection Delay (s)	9.3		
Level of Service	A	Level of Service	A		
Analysis Period (min)	15	Outflow of Service	0.69		

HCM Signalized Intersection Capacity Analysis		Year 2030 + Project AM 12/1/2010			
24: Carmel Grove Road & Carmel Creek Road					
Intersection Summary					
Delay					
HCM Level of Service	A	Intersection Capacity Utilization	0.69		
Analysis Period (min)	15	Outflow of Service	0.69		
Approach LOS	B	Approach LOS	B		
Approach Delay (s)	8.2	Approach Delay (s)	9.0		
Intersection LOS	A	Intersection LOS	B		
Intersection Delay (s)	9.3	Intersection Delay (s)	9.3		
Level of Service	A	Level of Service	A		
Analysis Period (min)	15	Outflow of Service	0.69		
Turn Types					
Protected Phases	4	Protected Phases	4		
Permitted Phases	4	Permitted Phases	4		
Actuated Green (s)	16.1	Actuated Green (s)	16.1		
Effective Green (s)	16.1	Effective Green (s)	16.1		
Actuated Cycle (s)	0.32	Actuated Cycle (s)	0.32		
Cycle Time (s)	4.0	Cycle Time (s)	4.0		
Vehicle Detection (s)	30.0	Vehicle Detection (s)	30.0		
Lane Gap Cap (vph)	500	Lane Gap Cap (vph)	500		
Yield Relieff (s)	0.10	Yield Relieff (s)	0.10		
Yield Perm	0.03	Yield Perm	0.03		
Yield Relieff	0.35	Yield Relieff	0.35		
Uniform Delay (s)	12.9	Uniform Delay (s)	11.9		
Progression Factor	0.76	Progression Factor	0.76		
Incremental Delay (s)	0.4	Incremental Delay (s)	0.4		
Delay (s)	13.3	Delay (s)	12.0		
Level of Service	B	Level of Service	B		
Approach Delay (s)	12.0	Approach Delay (s)	12.0		
Approach LOS	B	Approach LOS	B		
Intersection LOS	B	Intersection LOS	B		
Intersection Delay (s)	12.0	Intersection Delay (s)	12.0		
Level of Service	B	Level of Service	B		
Analysis Period (min)	15	Outflow of Service	0.69		
Critical Lane Group		Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
227: Valley Centre Drive & El Camino Real
Year 2030 + Project AM
12/1/2010

Year 2030 + Project AM
12/1/2010

HCM Signalized Intersection Capacity Analysis
28: Carmel Valley Road & El Camino Real

HCM Signalized Intersection Capacity Analysis
28: Carmel Valley Road & El Camino Real

Variable	Value	Description
Lane Configurations	0	Number of lanes in each direction.
Volume (v_h)	0	Peak-hour volume (veh/h).
Peak-hour Factor (PHF)	1.00	Ratio of peak-hour volume to average daily traffic (ADT).
Total LOS (LOS_t)	1.00	Total Lane Occupancy Score (LOS) for the entire network.
Lane Util. Factor	1.00	Factor applied to LOS to account for lane utilization.
Enbld. Pedestrian	1.00	Enbl. Pedestrian LOS component.
Fltr. Pedestrians	1.00	Fltr. Pedestrians LOS component.
Fltr. Vehicles	1.00	Fltr. Vehicles LOS component.
Fltr. Protected	0.95	Fltr. Protected LOS component.
Self-Filter (v_h)	0.95	Self-Filter LOS component.
Self-Filter (v_m)	0.95	Self-Filter LOS component.
PHF Permitted	0.95	PHF Permitted LOS component.
Self-Flow (v_m)	0.95	Self-Flow (v_m) LOS component.
Peak-hour factor PHF	0.90	Peak-hour factor PHF LOS component.
Avg. Flow (v_h)	0.90	Avg. Flow (v_h) LOS component.
RTOR. Reduction (vph)	0.90	RTOR. Reduction (vph) LOS component.
Lane Group Filter (v_h)	0.90	Lane Group Filter (v_h) LOS component.
Confl. Pets. (fltr.)	0.90	Confl. Pets. (fltr.) LOS component.
Fltr. Peds. (fltr.)	0.90	Fltr. Peds. (fltr.) LOS component.
Protected Phases	3	Number of protected phases.
Permitted Phases	3	Number of permitted phases.
Acculated Green, G_s	6	Acculated Green, G_s LOS component.
Elapsed Green, g_s	6	Elapsed Green, g_s LOS component.
Actuated g/C, Pello	0.25	Actuated g/C, Pello LOS component.
Clearance Time(s)	1.00	Clearance Time(s) LOS component.
Vehicle Extension(s)	3.0	Vehicle Extension(s) LOS component.
Lane Util. Gap (vph)	0.95	Lane Util. Gap (vph) LOS component.
vis Ratio Prol.	0.21	vis Ratio Prol. LOS component.
Vic Ratio	0.83	Vic Ratio LOS component.
Progression Factor	1.00	Progression Factor LOS component.
Integrated Delay (D)	7.0	Integrated Delay (D) LOS component.
Delay (s)	30.0	Delay (s) LOS component.
Length of Service	0.0	Length of Service LOS component.
Approach Delay (s)	0.0	Approach Delay (s) LOS component.
Approach LOS	0.0	Approach LOS component.

HCM Signalized Intersection Capacity Analysis
28: Carmel Valley Road & El Camino Real
Year 2030 + Project AM
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Westbound Lane 1: Left Turn onto Main Street									
Lane Configuration	WBL	WBBL							
Variance (Vt)	0	0	0	0	0	0	0	0	0
Total Flow (Vtph)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Cycle Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	1.00	0.97	0.95	0.97	0.95	0.96	0.96
Estimated Delays	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fltr. predators	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fltr. Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Self Selection (prob.)	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Selct. Flow (bent)	3.033	5.005	3.033	5.005	3.033	5.005	3.033	5.005	3.033
Peak-hour Factor PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Fltr. (Vtph)	0	0	0	0	0	0	0	0	0
RATOR Reduction (vph)	0	0	0	0	0	0	0	0	0
Lane Capacity (vph)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Confli. Peds. (fltr.)	0	0	0	0	0	0	0	0	0
Turn Type	Left Turn								
Protected Phases	3	6	5	2	6	6	6	6	6
Permitted Phases	1	1	1	1	1	1	1	1	1
Actualized Green (G_s)	16.0	16.0	16.0	9.6	40.4	26.8	26.8	26.8	26.8
Effective Green (g_e)	16.0	16.0	16.0	9.6	36.0	26.4	26.4	26.4	26.4
Actuated g/C Ratio	0.05	0.25	0.25	0.15	0.83	0.42	0.42	0.42	0.42
Gleason's Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane 1: Cap (vph)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
WBL Ratio	0.21	0.13	0.15	0.15	0.32	0.32	0.32	0.32	0.32
WBBL Ratio	0.03	0.54	0.59	0.87	0.56	0.77	0.99	0.77	0.99
VC Radio	23.9	21.0	21.3	26.8	6.9	16.1	18.1	16.1	18.1
WBBL Delay (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Progression Factor	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Interrelated Delay (s)	3.0	21.4	23.7	41.1	7.3	18.1	48.1	18.1	48.1
Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Length of Service	26.4	16.1	16.1	24.9	0.0	24.9	0.0	24.9	0.0
Approach Data (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Approach LOS	C	C	C	C	C	B	B	C	C

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Baseline

Synchro 7 - Report
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HCM Signalized Intersection Capacity Analysis
29: SR-56 EB on ramp & El Camino Real

Year 2030 + Project AM
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HCM Signalized Intersection Capacity Analysis
30: Valley Centre Drive & Camel View Road

Year 2030 + Project AM
12/1/10

Movement		EB		WB		EB		WB		EB		WB		EB		WB		EB	
Lane Configuration	Permitted Phases	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
Volume (vph)	800	930	310	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Identified Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util Factor	0.91	0.86	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Protected	0.95	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Salid Flow (vph)	1610	3182	1441	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PHF Permitted	0.95	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Salid Flow (vph)	1610	3182	1441	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	869	1033	344	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
RTHR Reduction (vph)	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	60	134	308	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Tun Type	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot
Rejected Phases	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	1	2	3	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Actuated Green, G (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Effective Green, g (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Actuated G/C Ratio	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	776	1524	694	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vs Ratio (vph)	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Vs Ratio, Perm	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21
Uniform Delay, d1	16.6	17.2	12.8	25.5	25.5	32.1	19.2	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5
Ingresson Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.1	5.3	0.5	0.8	0.8	23.7	2.1	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Delay (s)	23.0	22.4	13.2	25.3	25.3	35.5	2.1	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Level of Service	C	C	B	C	B	C	C	E	C	B	C	B	C	B	C	A	A	A	A
Approach Delay (s)	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5	21.5
Approach LOS	C	A	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C

Intersection Summary
HCM Average Control Delay 23.6 HCM Level of Service C
HCM Volume to Capacity ratio 0.83 Sum of lost time (s) 6.0
Adjusted Cycle Length (s) 74.7 HCM Level of Service G
Intersection Capacity Utilization 0.9534 HCM Level of Service G
Analysis Period (min) 15 Critical Lane Group C
C : Critical Lane Group

Movement		EB		WB		EB		WB		EB		WB		EB		WB		EB	
Lane Configuration	Permitted Phases	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
Volume (vph)	800	930	310	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Identified Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util Factor	0.91	0.86	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Protected	0.95	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Salid Flow (vph)	1610	3182	1441	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PHF Permitted	0.95	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Salid Flow (vph)	1610	3182	1441	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	869	1033	344	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
RTHR Reduction (vph)	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	60	134	308	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Tun Type	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot	Perm	Prot
Rejected Phases	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	1	2	3	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Actuated Green, G (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Effective Green, g (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Actuated G/C Ratio	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	776	1524	694	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vs Ratio (vph)	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Vs Ratio, Perm	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21	0.21
Uniform Delay, d1	16.6	17.2	12.8	25.5	25.5	32.1	19.2	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5
Ingresson Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.1	5.3	0.5	0.8	0.8	23.7	2.1	0.8	0.8	0.8	0								

HCM Signalized Intersection Capacity Analysis
31: Valley Centre Drive & Carmel Creek Road

Year 2030 + Project AM
12/12/2010

Lane Configurations	W	E	N	S	W	E	N	S	W	E	N	S	W	E	N	S	W	E	N	S	W	E	N	S	W	E	N	S	W	E	N	S				
Volume (vph)	65	220	180	300	555	195	380	571	384	146	394	220	395																							
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900						
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0					
Lane Util. Factor	1.00	0.88	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00						
Fl																																				
Fl Protected	0.96	1.00	0.96	1.00	0.95	1.00	0.96	1.00	0.95	1.00	0.96	1.00	0.95	1.00	0.96	1.00	0.95	1.00	0.96	1.00	0.95	1.00	0.96	1.00	0.95	1.00	0.96	1.00	0.95	1.00	0.96	1.00				
Sad. Flow (vph)	170	2767	1583	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822			
Fl Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00				
Fl Group Flow (vph)	170	2767	1583	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822	170	1822			
Turn Type	Proj	custom																																		
Projected Phases	4	4	6	6	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5				
Purified Phases																																				
Adjusted Green (G, s)	5.4	50.6	50.6	50.6	41.2	41.2	41.2	41.2	26.4	27.4	27.4	27.4	10.5	10.5	10.5	10.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5		
Effective Green (G, s)	5.4	50.6	50.6	50.6	41.2	41.2	41.2	41.2	26.4	27.4	27.4	27.4	10.5	10.5	10.5	10.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5	20.5		
Adjusted G/C Ratio	0.05	0.46	0.46	0.46	0.36	0.36	0.36	0.36	0.20	0.25	0.25	0.25	0.18	0.18	0.18	0.18	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			
Vehicle Detection (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Cap (vph)	87	1288	732	656	696	696	427	886	396	611	663	296																								
v/s Ratio Proj	0.03	0.19	0.13	0.50	0.39	0.39	0.39	0.39	25.0	25.0	25.0	25.0	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16			
v/s Ratio Pmt	0.09	0.06	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19		
Uniform Delay (s)	61.6	17.4	16.9	26.2	34.2	41.4	37.5	32.8	43.5	39.0	44.0																									
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay (s)	44.8	0.1	0.1	0.6	46.9	40.0	28	0.3	10.5	0.4	39.0																									
Delay (s)	85.4	17.4	66.9	26.8	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
Analysis Period (min)	F	B	B	C	F	F	F	F	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	
Approach LOS	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	E	

Intersection Summary	Year 2030 + Project AM	Baseline
HCM Average Control Delay	54.2	0
HCM Volume to Capacity ratio	1.00	1.00
Adjusted Cycle Length (s)	109.5	16.0
Intersection Capacity Utilization (%)	90.5%	15%
Analysis Period (min)	15	15
Critical Lane Group		

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HCM Signalized Intersection Capacity Analysis
34: SR-56 WB Ramp & Carmel County Road
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HCM Signalized Intersection Capacity Analysis
35: SR-56 EB Ramps & Carmel County Road
Year 2030 + Project AM
12/1/2010

HCM Signalized Intersection Capacity Analysis 35: SR-56 EB Ramps & Carmel County Road

Year 2030 + Project AM
12/1/2010

Traffic Flow Summary									
Lane Configurations	Volume (vh)	Capacity (vh)	Flow (vh)	Loss (vh)	Speed (km/h)	Time (s)	Distance (m)	Level of Service	Notes
Total Lost Time (s)	310	5	10	240	0	0	0	A	SWR=0.00
1900	1900	1900	1900	1900	1900	1900	1900	B	SWR=0.00
14.6	4.0	4.0	5.0	6.0	5.0	5.0	5.0	C	SWR=0.00
Lane Util. Factor	0.95	0.95	1.00	0.97	0.95	0.95	0.95	D	SWR=0.00
PH Factor	1.00	1.00	0.86	1.00	1.00	1.00	1.00	E	SWR=0.00
PH Factor (vh)	0.95	0.95	1.00	0.95	1.00	1.00	1.00	F	SWR=0.00
Saturation Factor (vh)	1.681	1.681	1.683	1.683	1.683	1.683	1.683	G	SWR=0.00
Lane Permitted (vh)	0.95	0.95	1.00	0.95	1.00	1.00	1.00	H	SWR=0.00
Sat. Factor (vh)	1.681	1.681	1.683	1.683	1.683	1.683	1.683	I	SWR=0.00
Peak-hour Factor, PHF	0.80	0.90	0.90	0.90	0.90	0.90	0.90	J	SWR=0.00
Adj. Flow (vh)	344	6	344	467	356	0	0	K	SWR=0.00
RTO Reduction (vh)	0	0	280	0	0	0	0	L	SWR=0.00
Lane Group Factor (vh)	175	175	175	64	167	356	0	M	SWR=0.00
Tun Type	Proj1	Proj2	Proj3	Proj4	Proj5	Proj6	Proj7	Proj8	Proj9
Projected Phases	1	2	3	4	5	6	7	8	9
Permitted Phases	1	2	3	4	5	6	7	8	9
Artificial Greening (s)	12.1	12.1	12.1	12.1	10.0	14.0	20.0	29.0	C
Electro Green, g (s)	12.1	12.1	12.1	12.1	10.0	44.0	29.0	29.0	C
Available Green (s)	0.19	0.19	0.19	0.19	0.16	0.68	0.16	0.44	C
Clearance Time (s)	4.0	4.0	4.0	4.0	5.0	5.0	6.0	5.0	C
Vehicle-to-Exterior (s)	30	30	30	30	30	30	30	30	C
Lane Seg Can (vh)	312	312	294	527	2392	1677	705	0	C
W/S Handoff (vh)	0.010	0.010	0.014	0.014	0.010	0.010	0.010	0.010	C
Vis Ratio Perm	0.55	0.55	0.55	0.55	0.55	0.55	0.55	0.55	C
Vis Ratio Actual	24.1	24.1	22.5	27.0	3.8	16.1	10.9	0	C
Uniform Delay, d1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	C
Progression Traffic	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	C
Incremental Delay, d2	2.3	2.3	0.4	16.3	0.0	4.4	0.1	0.1	C
Delay, d3	28.4	28.4	22.9	45.5	3.9	20.1	11.0	0.0	C
Level of Service	C	C	C	D	A	C	B	C	C
Approach LOS	C	C	C	C	C	B	B	B	C

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HCM Signalized Intersection Capacity Analysis
2: San Dieguito Road & El Camino Real
Year 2030 + Project PM
12/1/2010

HCM Signalized Intersection Capacity Analysis
3: Derby Downs Road & El Camino Real
Real ZUSU Project 1
12/1/2010

HCM Signalized Intersection Capacity Analysis
3: Derby Downs Road & El Camino Real

12/1/2010

12/1/2010

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在這裏，我們可以說，「人」是「社會」的「子」。

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HCM Signalized Intersection Capacity Analysis
6: Del Mar Heights Road & Mango Drive

Year 2030 + Project PM
12/1/2010

HCM Unsigned Intersections Capacity Analysis
7: Del Mar Heights Road & Portofino Drive

Year 2030 + Project PM
12/1/2010

Movement	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	990	991	992	993	994	995	996	997	998	999	1000	1001	1002	1003	1004	1005	1006	1007	1008	1009	1000	1001	100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HCM Signalized Intersection Capacity Analysis

Year 2030 + Project PM
4/22/2011

HCM Signalized Intersection Capacity Analysis
9: Del Mar Heights Rd. & I-5 SB Ramps

Lane Type	Protected Phases	Permitted Phases	Actuated Green, G (s)	Effective Green, g (s)	Actuated g/C Ratio	Progression Factor	Uniform Delay, d1	Incremental Delay, d2	Delay (s)	Level of Service	Approach Delay (s)	Approach LOS
Lane Gap Cap (vph)	2079	2079	1071	150	0.02	0.02	0.26	0.26	0.0	C	0.0	C
Vs Ratio Pct	0.76	0.72	1.00	0.94	0.94	0.94	1.23	1.18	1.08	0.94	1.05	F
Vc Ratio	1.23	1.18	27.5	25.6	0.40	0.40	48.0	25.5	39.5	38.4	38.5	F
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	F
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	108.8	28.8	46.9	49.4	117.0	F
Incremental Delay, d2	1.00	1.00	1.00	1.00	1.00	1.00	106.8	84.0	88.0	85.0	107.2	F
Delay (s)	1.7	1.2	28.1	12.6	0.0	0.0	14.1	13.0	55.6	38.2	116.5	D
Level of Service	B	B	E	D	0.0	0.0	B	B	F	F	F	A
Approach Delay (s)	14.1	13.0	50.9	14.1	0.0	0.0	14.1	13.0	50.9	14.1	120.0	F
Approach LOS	B	B	D	D	0.0	0.0	B	B	F	F	F	A

Year 2030 + Project PM
4/26/2011

HCM Signalized Intersection Capacity Analysis
9: Del Mar Heights Rd. & I-5 NB Ramps

Lane Type	Protected Phases	Permitted Phases	Actuated Green, G (s)	Effective Green, g (s)	Actuated g/C Ratio	Progression Factor	Uniform Delay, d1	Incremental Delay, d2	Delay (s)	Level of Service	Approach Delay (s)	Approach LOS
Lane Configuration	↑↑	↑↑	1891	0	0.0	0.0	1891	0	1697	753	630	F
Volume (vph)	1428	1344	0	931	400	400	1900	1900	1900	1900	1900	F
Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	F
Ideal Flow (vph)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	F
Total Lost time (s)	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	F
Lane Util. Factor	0.95	0.95	0.97	0.91	0.91	0.91	0.97	0.97	0.97	0.97	0.97	F
Filt.	1.00	1.00	0.99	0.85	0.85	0.85	1.00	1.00	1.00	1.00	1.00	F
Filt. (Blocked)	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	F
Sold. Flow (proj.)	3639	3539	3539	3539	3539	3539	3433	3539	3539	3605	1681	F
Filt. Permitted	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	F
Sold. Flow (perm.)	3639	3539	3539	3539	3539	3539	3433	3539	3539	3605	1681	F
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	F
Adj. EPF (vph)	1441	1441	1441	1441	1441	1441	1441	1441	1441	1441	1441	F
Adj. Flow (vph)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	F
Adj. TOR Reduction (vph)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	F
Lane Group Flow (vph)	1687	1493	1493	1493	1493	1493	1074	1074	1074	1074	1074	F
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	F
Protected Phases	6	6	6	6	6	6	6	6	6	6	6	F
Permitted Phases	6	6	6	6	6	6	6	6	6	6	6	F
Actuated Green, G (s)	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	F
Effective Green, g (s)	24.0	69.0	44.0	44.0	44.0	44.0	24.0	69.0	44.0	44.0	43.0	F
Actuated g/C Ratio	0.20	0.57	0.34	0.34	0.34	0.34	0.20	0.57	0.34	0.34	0.36	F
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	F
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	F
Lane Gap Cap (vph)	687	2035	1737	541	602	528	539	539	539	539	539	F
Vs Ratio Pct	0.24	0.59	0.37	0.37	0.37	0.37	0.24	0.59	0.37	0.37	0.44	F
Vc Ratio	1.21	1.03	1.03	1.03	1.03	1.03	1.21	1.03	1.03	1.03	1.03	F
Uniform Delay, d1	48.0	25.5	39.5	39.5	39.5	39.5	48.0	25.5	39.5	39.5	38.5	F
Incremental Delay, d2	108.8	28.8	46.9	46.9	46.9	46.9	108.8	28.8	46.9	46.9	47.0	F
Delay (s)	165.8	84.0	88.0	88.0	88.0	88.0	165.8	84.0	88.0	88.0	87.9	F
Level of Service	F	F	E	E	F	F	F	F	F	F	F	F
Approach Delay (s)	634	89.0	89.0	89.0	89.0	89.0	634	89.0	89.0	89.0	89.0	F
Approach LOS	F	F	F	F	F	F	F	F	F	F	F	F

Year 2030 + Project PM
4/26/2011

HCM Signalized Intersection Capacity Analysis
9: Del Mar Heights Rd. & I-5 NB Ramps

Lane Type	Protected Phases	Permitted Phases	Actuated Green, G (s)	Effective Green, g (s)	Actuated g/C Ratio	Progression Factor	Uniform Delay, d1	Incremental Delay, d2	Delay (s)	Level of Service	Approach Delay (s)	Approach LOS
Lane Configuration	↑↑	↑↑	1891	0	0.0	0.0	1891	0	1697	753	630	F
Volume (vph)	1428	1344	0	931	400	400	1900	1900	1900	1900	1900	F
Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	F
Ideal Flow (vph)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	F
Total Lost time (s)	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	F
Lane Util. Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	F
Filt.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	F
Filt. (Blocked)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	F
Sold. Flow (proj.)	3639	3539	3539	3539	3539	3539	3433	3539	3539	3605	1681	F
Filt. Permitted	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	F
Sold. Flow (perm.)	3639	3539	3539	3539	3539	3539	3433	3539	3539	3605	1681	F
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	F
Adj. EPF (vph)	1441	1441	1441	1441	1441	1441	1441	1441	1441	1441	1441	F
Adj. Flow (vph)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	F
Adj. TOR Reduction (vph)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	F
Lane Group Flow (vph)	1687	1493	1493	1493	1493	1493	1074	1074	1074	1074	1074	F
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	F
Protected Phases	6	6	6	6	6	6	6	6	6	6	6	F
Permitted Phases	6	6	6	6	6	6	6	6	6	6	6	F
Actuated Green, G (s)	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	141.0	F
Effective Green, g (s)	24.0	69.0	44.0	44.0	44.0	44.0	24.0	69.0	44.0	44.0	43.0	F
Actuated g/C Ratio	0.20	0.57	0.34	0.34	0.34	0.34	0.20	0.57	0.34	0.34	0.36	F
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	F
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	F
Lane Gap Cap (vph)	687	2035	1737	541	602	528	539	539	539	539	539	F
Vs Ratio Pct	0.24	0.59	0.37	0.37	0.37	0.37	0.24	0.59	0.37	0.37	0.44	F
Vc Ratio	1.21	1.03	1.03	1.03	1.03	1.03	1.21	1.03	1.03	1.03	1.03	F
Uniform Delay, d1	48.0	25.5	39.5	39.5	39.5	39.5	48.0	25.5	39.5	39.5	38.5	F
Incremental Delay, d2	108.8	28.8	46.9	46.9	46.9	46.9	108.8	28.8	46.9	46.9	47.0	F
Delay (s)	165.8	84.0	88.0	88.0	88.0	88.0	165.8	84.0	88.0	88.0	87.9	F
Level of Service	F	F	E	E	F	F	F	F	F	F	F	F
Approach Delay (s)	634	89.0	89.0	89.0	89.0	89.0	634	89.0	89.0	89.0	89.0	F
Approach LOS	F	F	F	F	F	F	F	F	F	F	F	F

Year 2030 + Project PM
4/26/2011

HCM Signalized Intersection Capacity Analysis
9: Del Mar Heights Rd. & I-5 NB Ramps

Lane Type	Protected Phases	Permitted Phases	Actuated Green, G (s)	Effective Green, g (s)	Actuated g/C Ratio	Progression Factor	Uniform Delay, d1	Incremental Delay, d2	Delay (s)	Level of Service	Approach Delay (s)	Approach LOS
Lane Configuration	↑↑	↑↑	1891	0	0.0	0.0	1891	0	1697	753	630	F
Volume (vph)	1428	1344	0	931	400	400	1900	1900	1900	1900	1900	F
Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	F
Ideal Flow (vph)	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0	F
Total Lost time (s)	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	F
Lane Util. Factor	0.95	0.95										

HCM Signalized Intersection Capacity Analysis
12: Del Mar Heights Road & First Ave.

Year 2030 + Project PM
12/1/2010

HCM Signalized Intersection Capacity Analysis
13: Del Mar Heights Road & El Camino Real

HCM Signalized Intersection Capacity Analysis
13: Del Mar Heights Road & El Camino Real
Year 2030 + Project H-1
12/1/2010

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HCM Signalized Intersection Capacity Analysis
14: Del Mar Heights Road & Carmel Country Rd.

Year 2030 + Project PM
12/1/2010

HCM Signalized Intersection Capacity Analysis
15; Del Mar Heights Road & Torrey Ridge Dr.

HCM Signalized Intersection Capacity Analysis
15; Del Mar Heights Road & Torrey Ridge Dr.
Year 2030 + Project PM
12/11/2010

Analysis Period (min) 15 Critical and Ground

HCM Signalized Intersection Capacity Analysis
15; Del Mar Heights Road & Torrey Ridge Dr.

HCM Signalized Intersection Capacity Analysis
15; Del Mar Heights Road & Torrey Ridge Dr.
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Analysis Period (min) 15 Critical Lane Group

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Baseline

HCM Signalized Intersection Capacity Analysis
16: Del Mar Heights Road & Lansdale Drive

Year 2030 + Project PM
12/1/2010

HCM Signalized Intersection Capacity Analysis
17: Del Mar Heights Road & Carmel Canyon Road

	Approach LOS	Approach Delay (s)	Approach Capacity (vph)	Approach Volume (vph)	Approach Lane Group Flow (vph)	Approach Lane Group Capacity (vph)	Approach Lane Group Utilization (%)	Approach Critical Lane Group	Approach Analysis Period (min)
Northbound	A	14.4	117	112	117	117	100	C	15
Southbound	B	14.4	117	112	117	117	100	C	15
Eastbound	C	14.4	117	112	117	117	100	C	15
Westbound	D	14.4	117	112	117	117	100	C	15
Total Lost Time (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Util. Factor		1.00	0.91	1.00	1.00	1.00	1.00		
Flt. Protected		1.00	0.99	1.00	0.99	1.00	0.99		
Flt. Permitted		1.00	0.95	1.00	0.95	1.00	0.95		
Flt. Total		1.00	0.95	1.00	0.95	1.00	0.95		
Peak hour factor, PHF		0.90	0.90	0.90	0.90	0.90	0.90		
Adj. Flow (vph)		347	152	72	41	770	308		
RTO/R Reduction (vph)		0	6	0	5	0	45		
Lane Group Flow (vph)		347	152	72	41	773	308		
Turn Type	Prot.					Prot.			
Protected Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Permitted Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Vehicle Extension (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Gap Cap (vph)		423	297	43	144	89	206		
Vs Ratio Prot.		50.20	39.37	37.01	34.16	30.04	31.03		
Vs Ratio Perm		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d1		22.3	11.6	29	19.1	29.6	22.3		
Vs Ratio		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d2		1.00	1.00	1.00	1.00	1.00	1.00		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay (s)		12.0	0.5	14.8	0.5	73.2	0.4		
Level of Service		C	B	D	B	F	C		
Approach Delay (s)		16.0	16.0	20.3	16.0	20.3	16.0		
Approach LOS		B	C	C	B	C	C		

Intersection Summary

HCM Average Control Delay

HCM Volume to Capacity Ratio

Actualized Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

Critical Lane Group

	Approach LOS	Approach Delay (s)	Approach Capacity (vph)	Approach Volume (vph)	Approach Lane Group Flow (vph)	Approach Lane Group Capacity (vph)	Approach Lane Group Utilization (%)	Approach Critical Lane Group	Approach Analysis Period (min)
Northbound	A	14.4	117	112	117	117	100	C	15
Southbound	B	14.4	117	112	117	117	100	C	15
Eastbound	C	14.4	117	112	117	117	100	C	15
Westbound	D	14.4	117	112	117	117	100	C	15
Total Lost Time (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Util. Factor		1.00	0.91	1.00	1.00	1.00	1.00		
Flt. Protected		1.00	0.99	1.00	0.99	1.00	0.99		
Flt. Permitted		1.00	0.95	1.00	0.95	1.00	0.95		
Flt. Total		1.00	0.95	1.00	0.95	1.00	0.95		
Peak hour factor, PHF		0.90	0.90	0.90	0.90	0.90	0.90		
Adj. Flow (vph)		347	152	72	41	770	308		
RTO/R Reduction (vph)		0	6	0	5	0	45		
Lane Group Flow (vph)		347	152	72	41	773	308		
Turn Type	Prot.					Prot.			
Protected Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Permitted Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Vehicle Extension (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Gap Cap (vph)		423	297	43	144	89	206		
Vs Ratio Prot.		50.20	39.37	37.01	34.16	30.04	31.03		
Vs Ratio Perm		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d1		22.3	11.6	29	19.1	29.6	22.3		
Vs Ratio		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d2		1.00	1.00	1.00	1.00	1.00	1.00		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay (s)		12.0	0.5	14.8	0.5	73.2	0.4		
Level of Service		C	B	D	B	F	C		
Approach Delay (s)		16.0	16.0	20.3	16.0	20.3	16.0		
Approach LOS		B	C	C	B	C	C		

Intersection Summary

HCM Average Control Delay

HCM Volume to Capacity Ratio

Actualized Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

Critical Lane Group

Intersection Summary

HCM Average Control Delay

HCM Volume to Capacity Ratio

Actualized Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

Critical Lane Group

	Approach LOS	Approach Delay (s)	Approach Capacity (vph)	Approach Volume (vph)	Approach Lane Group Flow (vph)	Approach Lane Group Capacity (vph)	Approach Lane Group Utilization (%)	Approach Critical Lane Group	Approach Analysis Period (min)
Northbound	A	14.4	117	112	117	117	100	C	15
Southbound	B	14.4	117	112	117	117	100	C	15
Eastbound	C	14.4	117	112	117	117	100	C	15
Westbound	D	14.4	117	112	117	117	100	C	15
Total Lost Time (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Util. Factor		1.00	0.91	1.00	1.00	1.00	1.00		
Flt. Protected		1.00	0.99	1.00	0.99	1.00	0.99		
Flt. Permitted		1.00	0.95	1.00	0.95	1.00	0.95		
Flt. Total		1.00	0.95	1.00	0.95	1.00	0.95		
Peak hour factor, PHF		0.90	0.90	0.90	0.90	0.90	0.90		
Adj. Flow (vph)		347	152	72	41	770	308		
RTO/R Reduction (vph)		0	6	0	5	0	45		
Lane Group Flow (vph)		347	152	72	41	773	308		
Turn Type	Prot.					Prot.			
Protected Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Permitted Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Vehicle Extension (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Gap Cap (vph)		423	297	43	144	89	206		
Vs Ratio Prot.		50.20	39.37	37.01	34.16	30.04	31.03		
Vs Ratio Perm		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d1		22.3	11.6	29	19.1	29.6	22.3		
Vs Ratio		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d2		1.00	1.00	1.00	1.00	1.00	1.00		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay (s)		12.0	0.5	14.8	0.5	73.2	0.4		
Level of Service		C	B	D	B	F	C		
Approach Delay (s)		16.0	16.0	20.3	16.0	20.3	16.0		
Approach LOS		B	C	C	B	C	C		

	Approach LOS	Approach Delay (s)	Approach Capacity (vph)	Approach Volume (vph)	Approach Lane Group Flow (vph)	Approach Lane Group Capacity (vph)	Approach Lane Group Utilization (%)	Approach Critical Lane Group	Approach Analysis Period (min)
Northbound	A	14.4	117	112	117	117	100	C	15
Southbound	B	14.4	117	112	117	117	100	C	15
Eastbound	C	14.4	117	112	117	117	100	C	15
Westbound	D	14.4	117	112	117	117	100	C	15
Total Lost Time (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Util. Factor		1.00	0.91	1.00	1.00	1.00	1.00		
Flt. Protected		1.00	0.99	1.00	0.99	1.00	0.99		
Flt. Permitted		1.00	0.95	1.00	0.95	1.00	0.95		
Flt. Total		1.00	0.95	1.00	0.95	1.00	0.95		
Peak hour factor, PHF		0.90	0.90	0.90	0.90	0.90	0.90		
Adj. Flow (vph)		347	152	72	41	770	308		
RTO/R Reduction (vph)		0	6	0	5	0	45		
Lane Group Flow (vph)		347	152	72	41	773	308		
Turn Type	Prot.					Prot.			
Protected Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Permitted Phases		17.0	17.0	17.0	17.0	17.0	17.0		
Vehicle Extension (s)		4.0	5.0	4.0	5.0	4.0	5.0		
Lane Gap Cap (vph)		423	297	43	144	89	206		
Vs Ratio Prot.		50.20	39.37	37.01	34.16	30.04	31.03		
Vs Ratio Perm		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d1		22.3	11.6	29	19.1	29.6	22.3		
Vs Ratio		0.02	0.01	0.02	0.01	0.02	0.01		
Uniform Delay, d2		1.00	1.00	1.00	1.00	1.00	1.00		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay (s)		12.0	0.5	14.8	0.5	73.2	0.4		
Level of Service		C	B	D	B	F	C		
Approach Delay (s)		16.0	16.0	20.3	16.0	20.3	16.0		
Approach LOS		B	C	C	B	C	C		

	Approach LOS	Approach Delay (s)	Approach Capacity (vph)	Approach Volume (vph)	Approach Lane Group Flow (vph)	Approach Lane Group Capacity (vph)	Approach Lane Group Utilization (%)	Approach Critical Lane Group	Approach Analysis Period (min)

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HCM Signalized Intersection Capacity Analysis
18: Del Mar Highlands Town Ctr. & El Camino Real

Year 2030 + Project PM
12/1/2010
19: Towngate Drive & Carmel County Road

	→	←	↑	↗	↖	↙	↘
Movement	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE
Lane Configurations	4↑1↓	4↑1↓	5↑1↓	5↑1↓	5↑1↓	5↑1↓	5↑1↓
Volume (vph)	295	51	228	37	301	84	166
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900
Total LOS time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	1.00	0.97	0.91	0.97	0.91	0.95
Filt.	1.00	1.00	0.98	0.98	1.00	0.98	1.00
Filt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (prot)	1681	1681	1786	1786	1683	1683	1683
Filt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (perm)	1681	1681	1786	1786	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	283	57	293	41	344	201	133
RTROR Reduction (vph)	0	0	0	0	25	0	20
Lane Group Flow (vph)	201	40	241	66	204	1295	0
Turn Type	Spill	Spill	Custom	Custom	Prot	Prot	Prot
Protected Phases	2	2	2	2	6	6	6
Permitted Phases	8	8	8	8	4	4	4
Actuated Green (G) (s)	11.6	11.6	11.6	11.6	16.8	15.2	27.7
Effective Green (g) (s)	14.9	14.9	14.9	14.9	16.8	25.2	11.8
Actuated g/C Ratio	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lane Gap Cap (vph)	298	292	394	474	377	1491	478
Vs Ratio Prot	0.12	0.12	0.16	0.16	0.16	0.11	0.13
Vs Ratio Perm	0.05	0.05	0.06	0.06	0.06	0.02	0.02
Vs Ratio	0.68	0.61	0.69	0.64	0.67	0.60	0.64
Uniform Delay, d1	32.7	32.2	32.6	22.3	35.7	26.3	35.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.1	3.8	16.1	0.2	1.6	0.1	0.2
Delay (s)	30.7	36.0	47.7	22.5	43.3	43.3	22.2
Level of Service	D	D	C	D	C	C	C
Approach LOS	D	C	C	C	C	C	C

	→	←	↑	↗	↖	↙	↘
HCM Average Control Delay	33.7	HCM Level of Service	C				
HCM Volume to Capacity ratio	0.80	Sum of lost time (s)	160				
Actuated Cycle Length (s)	84.7	ICU Level of Service	C				
Intersection Capacity Utilization	84.6%	Analysis Period (min)	15				
Critical Lane Group		Baseline					

	→	←	↑	↗	↖	↙	↘
Lane Configurations	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE
Volume (vph)	295	51	228	37	301	84	166
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900
Total LOS time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	1.00	0.97	0.91	0.97	0.91	0.95
Filt.	1.00	1.00	0.98	0.98	1.00	0.98	1.00
Filt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (prot)	1681	1681	1786	1786	1683	1683	1683
Filt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (perm)	1681	1681	1786	1786	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	283	57	293	41	344	201	133
RTROR Reduction (vph)	0	0	0	0	25	0	20
Lane Group Flow (vph)	201	40	241	66	204	1295	0
Turn Type	Spill	Spill	Custom	Custom	Prot	Prot	Prot
Protected Phases	2	2	2	2	6	6	6
Permitted Phases	8	8	8	8	4	4	4
Actuated Green (G) (s)	11.6	11.6	11.6	11.6	16.8	15.2	27.7
Effective Green (g) (s)	14.9	14.9	14.9	14.9	16.8	25.2	11.8
Actuated g/C Ratio	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lane Gap Cap (vph)	298	292	394	474	377	1491	478
Vs Ratio Prot	0.12	0.12	0.16	0.16	0.16	0.11	0.13
Vs Ratio Perm	0.05	0.05	0.06	0.06	0.06	0.02	0.02
Vs Ratio	0.68	0.61	0.69	0.64	0.67	0.60	0.64
Uniform Delay, d1	32.7	32.2	32.6	22.3	35.7	26.3	35.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.1	3.8	16.1	0.2	1.6	0.1	0.2
Delay (s)	30.7	36.0	47.7	22.5	43.3	43.3	22.2
Level of Service	D	D	C	D	C	D	C
Approach LOS	D	C	C	C	C	D	C

	→	←	↑	↗	↖	↙	↘
Lane Configurations	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE
Volume (vph)	295	51	228	37	301	84	166
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900
Total LOS time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	1.00	0.97	0.91	0.97	0.91	0.95
Filt.	1.00	1.00	0.98	0.98	1.00	0.98	1.00
Filt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (prot)	1681	1681	1786	1786	1683	1683	1683
Filt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (perm)	1681	1681	1786	1786	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	283	57	293	41	344	201	133
RTROR Reduction (vph)	0	0	0	0	25	0	20
Lane Group Flow (vph)	201	40	241	66	204	1295	0
Turn Type	Spill	Spill	Custom	Custom	Prot	Prot	Prot
Protected Phases	2	2	2	2	6	6	6
Permitted Phases	8	8	8	8	4	4	4
Actuated Green (G) (s)	11.6	11.6	11.6	11.6	16.8	15.2	27.7
Effective Green (g) (s)	14.9	14.9	14.9	14.9	16.8	25.2	11.8
Actuated g/C Ratio	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lane Gap Cap (vph)	298	292	394	474	377	1491	478
Vs Ratio Prot	0.12	0.12	0.16	0.16	0.16	0.11	0.13
Vs Ratio Perm	0.05	0.05	0.06	0.06	0.06	0.02	0.02
Vs Ratio	0.68	0.61	0.69	0.64	0.67	0.60	0.64
Uniform Delay, d1	32.7	32.2	32.6	22.3	35.7	26.3	35.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.1	3.8	16.1	0.2	1.6	0.1	0.2
Delay (s)	30.7	36.0	47.7	22.5	43.3	43.3	22.2
Level of Service	D	D	C	D	C	D	C
Approach LOS	D	C	C	C	C	D	C

	→	←	↑	↗	↖	↙	↘
Lane Configurations	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE
Volume (vph)	295	51	228	37	301	84	166
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900
Total LOS time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	1.00	0.97	0.91	0.97	0.91	0.95
Filt.	1.00	1.00	0.98	0.98	1.00	0.98	1.00
Filt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (prot)	1681	1681	1786	1786	1683	1683	1683
Filt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sold Flow (perm)	1681	1681	1786	1786	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	283	57	293	41	344	201	133
RTROR Reduction (vph)	0	0	0	0	25	0	20
Lane Group Flow (vph)	201	40	241	66	204	1295	0
Turn Type	Spill	Spill	Custom	Custom	Prot	Prot	Prot
Protected Phases	2	2	2	2	6	6	6
Permitted Phases	8	8	8	8	4	4	4
Actuated Green (G) (s)	11.6	11.6	11.6	11.6	16.8	15.2	27.7
Effective Green (g) (s)	14.9	14.9	14.9	14.9	16.8	25.2	11.8
Actuated g/C Ratio	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lane Gap Cap (vph)	298	292	394	474	377	1491	478
Vs Ratio Prot	0.12	0.12	0.16	0.16	0.16	0.11	0.13
Vs Ratio Perm	0.05	0.05	0.06	0.06	0.06	0.02	0.02
Vs Ratio	0.68	0.61	0.69	0.64	0.67	0.60	0.64
Uniform Delay, d1	32.7	32.2	32.6	22.3	35.7	26.3	35.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.1	3.8	16.1	0.2	1.6	0.1	0.2
Delay (s)	30.7	36.0	47.7	22.5	43.3	43.3	22.2
Level of Service	D	D	C	D	C	D	C
Approach LOS	D	C	C	C	C	D	C

	→	←	↑	↗	↖	↙	↘
Lane Configurations	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE	SW/NE
Volume (vph)	295	51	228	37</td			

HCM Signalized Intersection Capacity Analysis		Year 2030 + Project PM 12/1/2010	
22: High Bluff Drive & El Camino Real		23: High Bluff Drive & Carmel Vista Road	
Movement			
Lane Configurations	EBGD	EBGD	SWGD
Volume (Vph)	300	460	110
Peak Hour Flow (Vph)	1900	1900	1900
Total Lost time (s)	1.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.95
Sgt. Flow (vph)	1770	1583	1770
Flt Permitted	0.95	1.00	0.95
Sgt. Flow (vph)	1770	1583	1770
Peak-hour factor, P/H-F	0.90	0.90	0.90
Adj. Flow (vph)	89	333	544
R/D Reduction (vph)	0	0	244
Per Lane Group Flow (vph)	89	335	300
Turn Type	Perm	Perm	Perm
Pedestrian Phases	2	2	2
Authorized Green, G(s)	10.4	16.2	16.4
Effective Green, g(s)	16.4	18.4	18.4
Adjusted g/C Ratio	0.23	0.23	0.23
Change Time (s)	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0
Lane Gun Cap (vph)	410	367	367
W/Ratio Pmt	0.05	0.05	0.05
W/Ratio	0.22	0.9	0.8
Uniform Delay, d1	24.7	29.7	24.7
Progression Factor	1.00	1.00	1.00
Incremental Delay, d2	0.3	25.2	13.1
Delay (s)	24.9	51.6	42.1
Level of Service	C	D	D
Approach Delay (s)	45.0	45.0	45.0
Approach LOS	D	D	D
Intersection Summary			
HCM Average Control Delay	40.0	HCM Level of Service	
(VCM Volume / Capacity ratio)	0.86	D	
Actualized Cycle Length (s)	79.4	Sum of LOS time (s)	
Intersection Capacity Utilization	68.8%	ICU Level of Service	
Analysis Period (min)	15	D	
c : Critical Lane Group		Baseline	

HCM Unsigned Intersection Capacity Analysis		Year 2030 + Project PM 12/1/2010	
22: High Bluff Drive & El Camino Real		23: High Bluff Drive & Carmel Vista Road	
Movement			
Lane Configurations	SGGD	SGGD	SGGD
Volume (Vph)	1900	1900	1900
Peak Hour Flow (Vph)	1900	1900	1900
Total Lost time (s)	1.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.95
Sgt. Flow (vph)	1770	1583	1770
Flt Permitted	0.95	1.00	0.95
Sgt. Flow (vph)	1770	1583	1770
Peak-hour factor, P/H-F	0.90	0.90	0.90
Adj. Flow (vph)	89	333	544
R/D Reduction (vph)	0	0	244
Per Lane Group Flow (vph)	89	335	300
Turn Type	Perm	Perm	Perm
Pedestrian Phases	2	2	2
Authorized Green, G(s)	10.4	16.2	16.4
Effective Green, g(s)	16.4	18.4	18.4
Adjusted g/C Ratio	0.23	0.23	0.23
Change Time (s)	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0
Lane Gun Cap (vph)	410	367	367
W/Ratio Pmt	0.05	0.05	0.05
W/Ratio	0.22	0.9	0.8
Uniform Delay, d1	24.7	29.7	24.7
Progression Factor	1.00	1.00	1.00
Incremental Delay, d2	0.3	25.2	13.1
Delay (s)	24.9	51.6	42.1
Level of Service	C	D	D
Approach Delay (s)	45.0	45.0	45.0
Approach LOS	D	D	D
Intersection Summary			
HCM Average Control Delay	40.0	HCM Level of Service	
(VCM Volume / Capacity ratio)	0.86	D	
Actualized Cycle Length (s)	79.4	Sum of LOS time (s)	
Intersection Capacity Utilization	68.8%	ICU Level of Service	
Analysis Period (min)	15	D	
c : Critical Lane Group		Baseline	

HCM Unsigned Intersection Capacity Analysis		Year 2030 + Project PM 12/1/2010	
22: High Bluff Drive & El Camino Real		23: High Bluff Drive & Carmel Vista Road	
Movement			
Lane Configurations	SGGD	SGGD	SGGD
Volume (Vph)	1900	1900	1900
Peak Hour Flow (Vph)	1900	1900	1900
Total Lost time (s)	1.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.95
Sgt. Flow (vph)	1770	1583	1770
Flt Permitted	0.95	1.00	0.95
Sgt. Flow (vph)	1770	1583	1770
Peak-hour factor, P/H-F	0.90	0.90	0.90
Adj. Flow (vph)	89	333	544
R/D Reduction (vph)	0	0	244
Per Lane Group Flow (vph)	89	335	300
Turn Type	Perm	Perm	Perm
Pedestrian Phases	2	2	2
Authorized Green, G(s)	10.4	16.2	16.4
Effective Green, g(s)	16.4	18.4	18.4
Adjusted g/C Ratio	0.23	0.23	0.23
Change Time (s)	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0
Lane Gun Cap (vph)	410	367	367
W/Ratio Pmt	0.05	0.05	0.05
W/Ratio	0.22	0.9	0.8
Uniform Delay, d1	24.7	29.7	24.7
Progression Factor	1.00	1.00	1.00
Incremental Delay, d2	0.3	25.2	13.1
Delay (s)	24.9	51.6	42.1
Level of Service	C	D	D
Approach Delay (s)	45.0	45.0	45.0
Approach LOS	D	D	D
Intersection Summary			
HCM Average Control Delay	40.0	HCM Level of Service	
(VCM Volume / Capacity ratio)	0.86	D	
Actualized Cycle Length (s)	79.4	Sum of LOS time (s)	
Intersection Capacity Utilization	68.8%	ICU Level of Service	
Analysis Period (min)	15	D	
c : Critical Lane Group		Baseline	

Year 2030 + Project PM
12/1/2010
HCM Signalized Intersection Capacity Analysis
24: Carmel Grove Road & Carmel Creek Road

Year 2030 + Project PM
12/1/2010

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HCM Signalized Intersection Capacity Analysis
25: Carmel Valley Road & I-5 SB Ramps

HCM Signalized Intersection Capacity Analysis

23. Cartier Valley Road & F-3 3B Train

Year 2030 + Project PM
12/1/2010

Year 2030 + Project PM
12/1/2010

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Lane Configurations		HCM Average Control Delay		HCM Level of Service	
Volume (vph)	Flow (vph)	HCM Volume of Traffic	Controlled Flow (vph)	Sum of lost time (s)	LOS Level of Service
Total Lost Time (s)					
Lane Util. Factor					
Friction Factor					
FII Protected					
Scal. FII (vph)					
FII Permitted					
Scal. FII (vph)					
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	0.91	500	722	1041	190
RITOR Reduction (vph)	0	74	0	0	0
Lane Control Flow (vph)	0	607	0	0	0
Turn Type					
Protected Phases					
Permitted Phases					
Relative Green (%)	57.0	33.3	20.7	62.7	19.0
Effective Green (s)	37.0	20.7	62.7	19.0	18.0
Abandon & Recall	0.41	0.33	0.30	0.52	0.21
Clearance Time (s)	6.0	4.0	4.0	4.0	5.0
Vehicle Extension (s)	1.0	0.3	0.3	3.0	3.0
Lane End Cap (vph)	1385	792	2474	356	338
vs. Ratio Perm.	0.4	0.2	0.2	0.12	0.13
Yield Ratio	1.00	0.91	0.92	0.98	0.91
Uniform Delay, d1	26.4	33.6	5.0	31.8	31.9
Programmed Factor	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.5	14.7	0.1	2.3	2.8
Delay (s)	500	333	9.9	34.6	34.6
Level of Service	D	D	C	C	C
Approach Delay (s)	500	333	25.2	0.0	330
Approach LOS	D	C	A	A	C

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HCM Signalized Intersection Capacity Analysis
30: Valley Centre Drive & Carmel View Road

Year 2030 + Project PM
12/1/2010

HCM Signalized Intersection Capacity Analysis
31: Valley Centre Drive & Carmel Creek Road

Year 2030 + Project PM
12/1/2010

	Year 2030 + Project PM		Year 2030 + Project PM	
	12/1/2010	12/1/2010	12/1/2010	12/1/2010
Intersection Summary				
Lane Configurations	A	B	C	D
Vehicle/veh	175	441	0	107
Vehicle/veh	1910	1900	1900	1900
Total Flow (vph)	1.00	1.00	1.00	1.00
Total Lost time (s)	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00
Filt.	1.00	1.00	1.00	1.00
Filt Protected	0.95	1.00	1.00	1.00
Sad. Flow (prot)	1770	3598	0	1770
Filt Permitted	0.95	1.00	1.00	1.00
Sad. Flow (perm)	1770	3596	0	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90
Adj. Flow (vph)	163	490	0	163
RTOR Reduction (vph)	0	0	0	0
Lane Group Flow (vph)	63	450	0	529
Lane Group Cap (vph)	143	2055	0	1187
Lane Group LOS	A	A	B	B
Approach LOS	A	A	A	B
HCM Level of Service				
HCM Average Control Delay	6.2	A		
HCM Volume to Capacity Ratio	0.41			
Actualized Cycle Length (s)	12.0			
Intersection Capacity Utilization (%)	85.5			
Analysis Period (min)	15			
Critical Lane Group	c			

	Year 2030 + Project PM		Year 2030 + Project PM	
	12/1/2010	12/1/2010	12/1/2010	12/1/2010
Lane Configurations				
Véhicle/veh	142	300	130	150
Ideal Flow (vph)	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00
Filt.	1.00	1.00	1.00	1.00
Filt Protected	0.95	1.00	1.00	1.00
Sad. Flow (prot)	1770	2767	1563	1770
Filt Permitted	0.95	1.00	1.00	1.00
Sad. Flow (perm)	1770	2767	1563	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90
Adj. Flow (vph)	124	353	167	176
RTOR Reduction (vph)	0	0	0	0
Lane Group Flow (vph)	124	333	62	167
Turn Type	Prot	custom	Prot	Prot
Projected Phases	17	3	5	6
Permitted Phases	4	4	2	2
Adjusted Green, G(s)	16.5	32.5	10.5	13.5
Effective Green, g(s)	8.4	18.5	13.2	23.3
Adjusted g/C Ratio	0.68	0.17	0.77	0.21
Clearance Time (s)	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	135	469	266	212
VS Ratio Prot	0.07	0.09	0.22	0.25
VS Ratio Perm	0.12	0.04	0.14	0.16
VS Ratio C	0.92	0.95	0.9	0.93
Uniform Delay, d1	50.5	43.2	39.6	47.0
Progression Factor	1.00	1.00	1.00	1.00
Incremental Delay, d2	52.8	5.0	0.4	17.4
Delay (s)	100	100	100	100
Level of Service	F	D	E	F
Approach Delay (s)	85.3	57.2	50.9	50.0
Approach LOS	F	D	C	D
Intersection Summary				
HCM Average Control Delay	63.3	D		
HCM Volume to Capacity Ratio	0.50			
Actualized Cycle Length (s)	8.0			
Intersection Capacity Utilization (%)	86.5			
Analysis Period (min)	15			
Critical Lane Group	c			

HCM Signalized Intersection Capacity Analysis
34: SR-56 WB Ramps & Carmel County Road
Year 2030 + Project PM
12/1/2010

HCM Signalized Intersection Capacity Analysis
35: SR-56 EB Ramps & Carmel County Road
Year 2030 + Project PW
12/1/2010

Intersection Summary		Network Summary	
Lane Configurations	4 Left Turn Lanes, 2 Through Lanes, 2 Right Turn Lanes	Number of Intersections	5
Volume (vph)	0	Approach LOS	A
Ideal Flow (vph)	1900	HCM Average Control Delay	127
Total Lost Time (s)	1900	HCM Volume to Capacity Ratio	0.5
Lane Util. Factor	4.0	Actualized Cycle Length (s)	19.1
Fit	0.97	Sum of Lost Time (s)	49.7
Fit Predicted	1.00	Intersection Capacity Utilization (%)	77.8%
Sad, Fitby (prot)	0.95	Analysis Period (min)	15
Fit Parallel	0.95	Critical Area Factor	A
Sad, Flow (perf)	0.95	Approach LOS	B
Peak-hour factor, PHF	0.90	Intersection Summary	B
Adj. Flow (vph)	900	HCM Level of Service	B
RTOR Reduction (vph)	0	Sum of Lost Time (s)	12.0
Lane Group Flow (vph)	0	Intersection Capacity Utilization (%)	A
Turn Type	Prior	Analysis Period (min)	15
Projected Phases		Permit	
Permitted Phases		Phase 1	4
Effective Green, G (s)	103	Phase 2	6
Adjusted Green (s)	103	Phase 3	8
Adjusted G/Ratio	0.21	Phase 4	4
Clearance Time (s)	40	Phase 5	6
Vehicle Extension (s)	50	Phase 6	6
Lane Gap Cap (vph)	711	Phase 7	6
W/Ratio (Per Lane)	0.13	Phase 8	6
W/Ratio (Total)	0.67	Phase 9	0.05
Net Ratio (Per Lane)	17.9	Phase 10	0.07
Net Ratio (Total)	10.0	Phase 11	0.07
Uniform Delay, d1	12.8	Phase 12	0.07
Progression Factor	1.00	Phase 13	0.07
Incremental Delay, d2	0.1	Phase 14	0.07
Delay (s)	1.3	Phase 15	0.07
Level of Service	A	Phase 16	0.07
Approach LOS	A	Phase 17	0.07
Intersection Summary	B	HCM Level of Service	B
HCM Average Control Delay	127	Sum of Lost Time (s)	12.0
HCM Volume to Capacity Ratio	0.5	Intersection Capacity Utilization (%)	A
Actualized Cycle Length (s)	19.1	Analysis Period (min)	15
Intersection Capacity Utilization (%)	77.8%	Critical Area Factor	A
Analysis Period (min)	15	Approach LOS	B
Critical Area Factor	A	Intersection Summary	B

HCM Signalized Intersection Capacity Analysis
35; SR-56 EB Ramps & Carmel Country Road

HCM Signalized Intersection Capacity Analysis
35: SR-56 EB Ramps & Carmel County Road
Year 2030 + Project PW
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ALL-WAY STOP CONTROL ANALYSIS

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