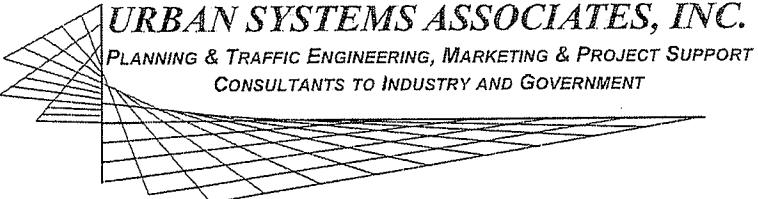


APPENDIX O

CONSTRUCTION TRAFFIC IMPACT ANALYSIS



ATTN:

Bob Little – Kilroy Realty

E-Mail: ▼

rlittle@kilroyrealty.com

FROM: *Andrew P. Schlaefli & Jacob D. Swim*

TOTAL PAGES (Including 5 + (40)
Cover): Attachments

DATE: *March 5, 2012*

TIME: *9:22:42 AM* JOB NUMBER: *002407*

SUBJECT: *One Paseo - Construction Traffic Analysis*

Confidential Communications

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INTRODUCTION AND METHODOLOGY:

The purpose of this memo is to discuss the impacts of construction traffic as a result of the proposed One Paseo project. A total of five construction phases were evaluated for traffic impacts which included Phase 1, Phase 2, Phase 3, Phase 1&2, and Phase 1-3. Each construction phase is consistent with the project phasing outlined in the Development Summary, see **Attachment 1**. Phase 1&2 indicates these two phases would be constructed together. Phase 1-3 implies all three construction phases would be combined. Two scenarios were evaluated for each phase: Existing with Construction Traffic and Near Term with Construction Traffic. The existing with construction traffic scenario evaluates the baseline condition with construction traffic by phase. And the Near Term with construction traffic evaluated existing with cumulative projects in the area along with construction traffic by phase. These two scenarios were analyzed based on the assumption the proposed project would begin construction of Phase 1 in 2013, Phase 2 would begin in 2014, and Phase 3 would begin in 2015. Existing with construction traffic by phase and Near Term with construction traffic by phase was analyzed on seven (7) street segments, five (5) intersections, and two (2) freeway segments. The study area for the construction analysis was based on the assumed truck routes exporting and importing materials to the site via Del Mar Heights Road and Interstate 5. The construction phasing, duration of construction, workers, deliveries, and import/export by phase was provided by Whiting-Turner, see attached email dated October 20, 2011. **Attachment 2** shows the proposed employee parking, construction trailers, truck routes and load sites. As shown, trucks would turn right into the site at Third Avenue and then exit at the signalized access at First Avenue.

A distribution of construction traffic for employees is shown on **Attachment 3**. The distribution is based on our experience with similar projects and familiarity with the Carmel Valley area. As shown, 60% of employee traffic is assumed to be coming from the west on Del Mar Heights Road. This assumption is based on the close proximity the I-5 freeway is to the project site. 50% (25% north and 25% south) of the construction employees are assumed to use the I-5 freeway to access the project since there are many residential communities both north and south of the Del Mar Heights Road interchange. Just east of First Avenue at Del Mar Heights Road, 40% of construction employees are assumed to enter/exit the site. Of the 40%, 10% come from the east on Del Mar

Heights Road, 5% from the north on El Camino Real, and 25% from the south on El Camino Real. These employees are assumed to be already in the Carmel Valley area or nearby surrounding communities.

Attachment 4 illustrates the distribution of truck (import/export) and delivery traffic on a daily basis. Based on our experience with similar construction projects, the haul routes are designed to access nearby material sources (mines). The project site is located in a primarily residential and commercial area where no mines are located. Based on location of nearby mines in the San Diego area, the I-5 would be the most likely route to the project site. As shown in **Attachment 4**, 100% of the truck & delivery traffic is assumed to use Del Mar Heights Road from the west and then 50% travel north and 50% south on Interstate 5. To accommodate trucks leaving the site, a signal is proposed at First Avenue and Del Mar Heights Road for construction access. Without a signal at Third Avenue, a dedicated right turn lane in the eastbound direction would provide trucks and delivery vehicles a deceleration lane to minimize stops and enhance traffic safety during construction along the Del Mar Heights corridor.

To determine Near Term traffic impacts during construction, a percentage of cumulative projects by phase were assumed as shown in **Attachment 5**. The project's trip generation daily traffic was broken down by phase to determine the amount of cumulative projects built by a particular phase. For example, the project development in Phase 1 generates 9,888 average daily trips (ADT) which is approximately 37% of the projects total 26,961 (Project ADT). Therefore, in Phase 1, an assumed 37% of cumulative projects in the area would be constructed and included in the roadway network. For Phase 2, an assumed 66% of cumulative project would be built. Phase 3 assumes 100% of cumulative projects in the area are built.

If construction traffic causes a roadway facility or intersection that operates acceptably to operate unacceptably, then the project has a significant impact. Two criteria must be met to determine a significant impact and before mitigation is proposed. First, the intersection or street segment must have an unacceptable level of service (LOS), i.e. E or F. Second, the amount of construction traffic must be significant based on the application of criteria discussed below and illustrated in Table 4-2 of the traffic study. For an intersection, if the change in delay is greater than 2 seconds or 1 second and the level of service is "E" or "F" respectively, then the intersection construction impacts would be considered significant. For a street segment, if the change in volume to capacity ratio (V/C ratio) exceeds 0.02 or 0.01, and the level of service is "E" or "F" respectively, then the street segment would be considered significantly impacted.

CONSTRUCTION TRAFFIC (PHASE 1):

Attachment 6 shows the trip generation table for traffic generated by employees, material deliveries, and trucks importing and exporting in phase 1. Construction traffic in this phase would generate 1,775 ADT with 130 AM peak hour trips and 118 PM peak hour trips. The material deliveries and truck imports/exports are converted from trucks to autos using an auto equivalency of 2.5 per Exhibit 21-9 in the Highway Capacity Manual 2000.

Existing With Construction Traffic (Phase 1):

The Existing with construction traffic in phase 1 was evaluated. **Attachment 7** shows the Existing with and without construction street segment comparison in phase 1. As shown, all segments operate at acceptable levels of service, i.e. D or better. **Attachment 8** shows the Existing with and without construction intersection comparison in phase 1. Five (5) intersections were evaluated on Del Mar Heights Road from the I-5 SB ramps to El Camino Real. As shown in the table, all intersections operate at acceptable levels of service and no

significant impacts occur as a result of construction traffic in phase 1. The freeway segments north and south of Del Mar Heights Road on Interstate 5 were evaluated and operate at acceptable levels of service as shown in the freeway level of service summary table, see **Attachment 9**. As shown, no impacts occur as a result of construction traffic in phase 1.

Near Term With Construction Traffic (Phase 1):

The Near Term without construction traffic volumes in phase 1 were developed by assuming 37% of cumulative projects were built. Construction traffic volumes in phase 1 were then added to Near Term volumes to derive the Near Term with construction traffic volumes and evaluate construction traffic impacts. The Near Term street segment comparison table shows no significant impacts, see **Attachment 10**. For intersection impacts, **Attachment 11** shows all intersections are projected to operate at acceptable levels of service with and without construction traffic in phase 1, therefore, no significant impacts. On the I-5 freeway segments analyzed, **Attachment 12** shows no significant impacts as a result of construction traffic in phase 1.

CONSTRUCTION TRAFFIC (PHASE 2):

The construction traffic in phase 2 generates 1,265 ADT with 84 AM peak hour trips and 77 PM peak hour trips, see **Attachment 13**.

Existing With Construction Traffic (Phase 2):

Street segment volumes for construction traffic in phase 2 were added to existing traffic volumes and evaluated. As shown in **Attachment 14**, all segments evaluated operate at level of service "D" or better. Intersections were analyzed and there are no significant impacts as a result of construction traffic in phase 2, see **Attachment 15**. The intersection of Del Mar Heights Road at Third Avenue is analyzed based on construction of Block A being part of phase 2. A freeway level of service summary shows both segments on I-5 operate at acceptable levels of service and there are no significant impacts as shown in **Attachment 16**.

Near Term With Construction Traffic (Phase 2):

The Near Term with construction traffic volumes were developed by adding the Near Term volumes (Existing + 66% of cumulative projects) to the construction traffic volumes in phase 2. The street segment level of service comparison shows no significant street segment impacts, see **Attachment 17**. The intersection comparison table shows all intersections operating at acceptable levels of service and therefore, no significant impacts as shown in **Attachment 18**. The freeway segments on I-5 are operating at acceptable levels of service with no significant impacts, see **Attachment 19**.

CONSTRUCTION TRAFFIC (PHASE 3):

The construction traffic in phase 3 generates 1,369 ADT with 93 AM peak hour trips and 86 PM peak hour trips, see **Attachment 20**.

Existing With Construction Traffic (Phase 3):

The Existing with construction traffic in phase 3 was derived by adding construction traffic volumes in phase 3 to existing volumes. All street segments in **Attachment 21** currently operate at acceptable levels of service with and without construction traffic in phase 3. **Attachment 22** shows all intersections operating at acceptable levels of service and no significant impacts. The two segments on I-5 in phase 3 of construction operate at acceptable levels of service and show no significant impacts in **Attachment 23**.

Near Term With Construction Traffic (Phase 3):

The Near Term without construction traffic volumes in phase 3 assumed 100% of cumulative projects in the area were built. Construction traffic volumes from phase 3 were added to Near Term without construction traffic volumes to get the Near Term with construction traffic volumes. **Attachment 24** shows all street segments operate at level of service "D" or better. As shown on **Attachment 25**, there are no significant impacts at intersections as a result of construction traffic in phase 3. Freeway segments are operating at acceptable levels of service as shown in **Attachment 26**.

CONSTRUCTION TRAFFIC (PHASE 1 & 2):

As previously mentioned, construction traffic in phase 1 & 2 assumes these phases are built together. Phase 1 & 2 includes the construction of Blocks D, E, and A as shown in the Development Summary Table (**Attachment 1**). Since the construction of Block A is adjacent to the First Avenue entrance, it is assumed in this analysis that a signal is installed at First Avenue for construction access under this phasing scenario. **Attachment 27** shows the trip generation table for construction phase 1 & 2. As shown, the construction traffic would generate 1,975 ADT with 138 AM peak hour trips and 126 PM peak hour trips.

Existing With Construction Traffic (Phase 1 & 2):

The existing with and without construction traffic street segment comparison table for phase 1 & 2 shows no unacceptable level of service on any street segments, therefore, no significant segment impacts, see **Attachment 28**. **Attachment 29** shows the intersection comparison table for construction phase 1 & 2. As shown, there are no significant impacts to intersections evaluated. Freeway segments operate at acceptable levels of service, see **Attachment 30**.

Near Term With Construction Traffic (Phase 1 & 2):

The Near Term without construction traffic volumes in phase 1 & 2 were developed by assuming 66% of cumulative projects were built. The Near Term street segment comparison table shows all segments operate at level of service "D" or better, and therefore, no significant impacts, see **Attachment 31**. For intersection impacts, **Attachment 32** shows all intersections are projected to operate at acceptable levels of service, i.e. D or better, with and without construction traffic in phase 1 & 2, therefore, no significant impacts. On the I-5 freeway segments analyzed, **Attachment 33** shows no significant impacts as a result of construction traffic in phase 1 & 2.

CONSTRUCTION TRAFFIC (PHASE 1 - 3):

Construction traffic in phases 1-3 assumes all three phases are built together, not sequentially. This phasing assumes the intersection of First Avenue and Del Mar Heights Road is signalized for construction access. The construction traffic in phases 1-3 generates 2,175 ADT with 146 AM peak hour trips and 134 PM peak hour trips, see **Attachment 34**.

Existing With Construction Traffic (Phase 1 - 3):

Attachment 35 shows no significant impacts in the street segment comparison table. No significant intersection impacts occur in phases 1-3 as a result of construction traffic, see **Attachment 36**. On the I-5 freeway segments, **Attachment 37** shows no significant impacts.

Near Term With Construction Traffic (Phase 1 - 3):

The Near Term analysis for construction traffic in phases 1-3 assumes 100% cumulative projects are built and included in the roadway network. A significant impact occurs on Del Mar Heights Road between I-5 NB ramps and High Bluff Drive in the street segment comparison table as shown in **Attachment 38**. All intersections are projected to operate at acceptable levels of service in **Attachment 39**. **Attachment 40** shows no significant impacts to freeway segments as a result of construction traffic from all three construction phases.

CONCLUSIONS AND RECOMMENDATIONS:

The only construction traffic significant impact is a segment impact on Del Mar Heights Road between the I-5 NB ramps and High Bluff Drive. This significant impact on Del Mar Heights Road occurs under the analyzed construction (Phase 1-3) scenario. A significant impact resulting from project construction traffic only occurs if the additional trips cause the segment to exceed the City's significance thresholds and the segments operates at an unacceptable level of service.

ATTACHMENT 1

ONE PASEO – A Main Street for Carmel Valley
DEVELOPMENT SUMMARY

Phase/Block	Commercial Retail (Sq. Ft.*)		Commercial Office (Sq. Ft.*)		Hotel (No. of Rooms)	Residential (MF Units)	Total*
	Retail	Cinema **	Corporate Office	Professional Office ***			
<i>Phase 1</i>							
Block D	61,190	—	270,000	21,000	—	—	352,190
Block E	39,460	—	245,000	—	—	—	284,460
<i>Phase 1 Total</i>	<i>100,650</i>	—	<i>515,000</i>	<i>21,000</i>	—	—	<i>636,650</i>
<i>Phase 2</i>							
Block A	65,610	—	—	—	—	194	65,610 + 194 MF units
<i>Phase 2 Total</i>	<i>65,610</i>	—	—	—	—	194	<i>65,610 + 194 MF units</i>
<i>Phase 3</i>							
Block B	38,940	—	—	—	150	181	38,940 + 150 hotel rooms + 181 MF units
Block C	14,890	—	—	—	—	233	14,890 + 233 MF units
Block D	—	50,000	—	—	—	—	50,000
<i>Phase 3 Total</i>	<i>53,740</i>	<i>50,000</i>	—	—	—	—	<i>103,740 + 418 MF units</i>
Total*	220,000	50,000	515,000	21,000	150	608	806,000 Sq. Ft + 150 hotel rooms + 608 MF units

*Gross Leasable Area (excludes parking structures covered in Gross Floor Area calculations). Density transfers permitted in accordance with procedures described in the Precise Plan.

** Cinema consists of up to 10 screens with a maximum total of 1,200 seats.

*** Professional Office (located on Main Street).

Jake Swim

From: Likins, Steven [Steven.Likins@Whiting-Turner.com]
 Sent: Thursday, October 20, 2011 10:17 AM
 To: jake@urbansystems.net
 Subject: Construction Analysis

Jake –

Per your June 24, 2011 memo, please see our responses below.

- Construction Phases - The project is divided into three phases
- Construction Duration by Phase - Phase I (28 months), Phase II (22 months) & Phase III (31 months). The project would take a total of 81 months to build, if phases are built sequentially. If Phase I and II were combined, it would also take 28 months. If all phases were combined, it would take 40 months to complete.
- Number of Workers by Phase - Whiting-Turner estimated that a project built sequentially would likely have a maximum of 400 employees per day and generally average approximately 300 employees per day. If Phase I and II were combined, they would likely have a maximum of 700 employees per day and generally average approximately 400 employees per day. If all phases were combined, they would likely have a maximum of 800 employees and generally average approximately 500 employees per day.
- Number of Material Deliveries by Phase - On average, the project would result in 25 deliveries per day. The peak would occur during export operation, but during the export operation there won't be many workers on the site.
- Amount of Import & Export by Phase - Phase I-243,670 cu. yds., Phase II - 118,800 cu. yds. and Phase III - 141,500 cu.yds. Each truck trip can accommodate 10 cu. yds. per trip. As an example, Phase 1 export (i.e. trucks on the street) is 110 days (22 weeks x5 days) or roughly 210 round trips per day (24,260 trips / 110 days). Assuming a round trip is about 20 minutes (depends on distance to export site) there would be about 9-10 trucks running round trips for 110 work days (i.e. Mon-Fri). If you combine Phases I and II, that would add another 9-10 trucks for about 60 days. And, If all three phases are combined, Phase 3 export would occur after phase 1 and 2 so we'd still be doing 210 trips per day (420 for 55 of those while doing phase 2) but we'd be adding 55 days of export.
- Construction Hours - Monday-Friday (The Municipal Code allows from 7:00 AM-7:00 PM but these hours may be further restricted by the site development permit).
- Construction Shifts, if any - None anticipated
- Written Description of construction activities by phase - Each phase is basically the same; shoring, excavation, site utilities, below grade parking garage, vertical steel, concrete or wood structural frame, glass, plaster, metal panel or stone exterior enclosure, HVAC, Plumbing, Electrical, interior drywall and finishes (floor finishes, ceilings, walls, doors) exterior hardscape and landscape

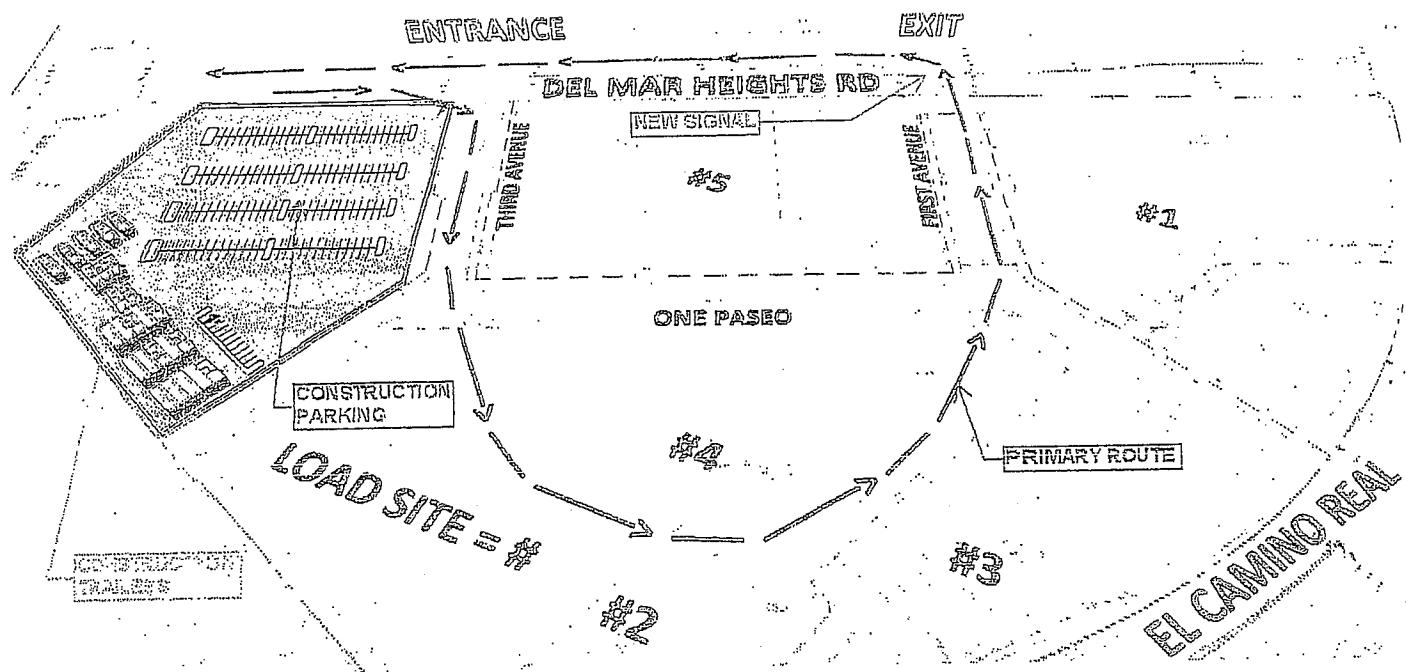
Please advise if you have any questions.

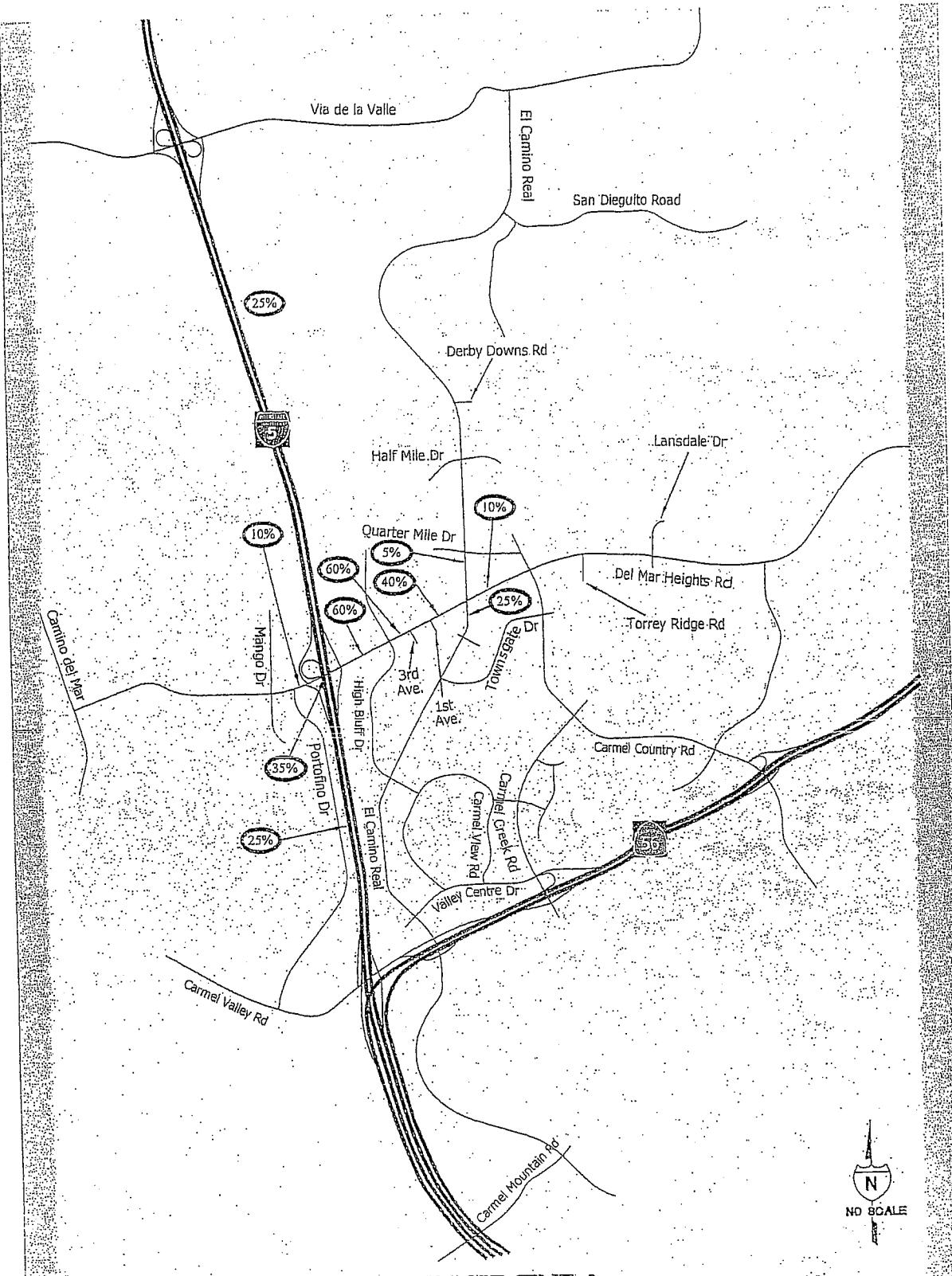
Thanks

Steve Likins | Vice President
THE WHITING-TURNER CONTRACTING COMPANY
 San Diego Office | 440 Stevens Avenue Suite 270 Solana Beach CA 92075
 T: (858) 792-0600 | F: (858) 792-9600 | C: (949) 289-7492
www.whiting-turner.com

10/20/2011

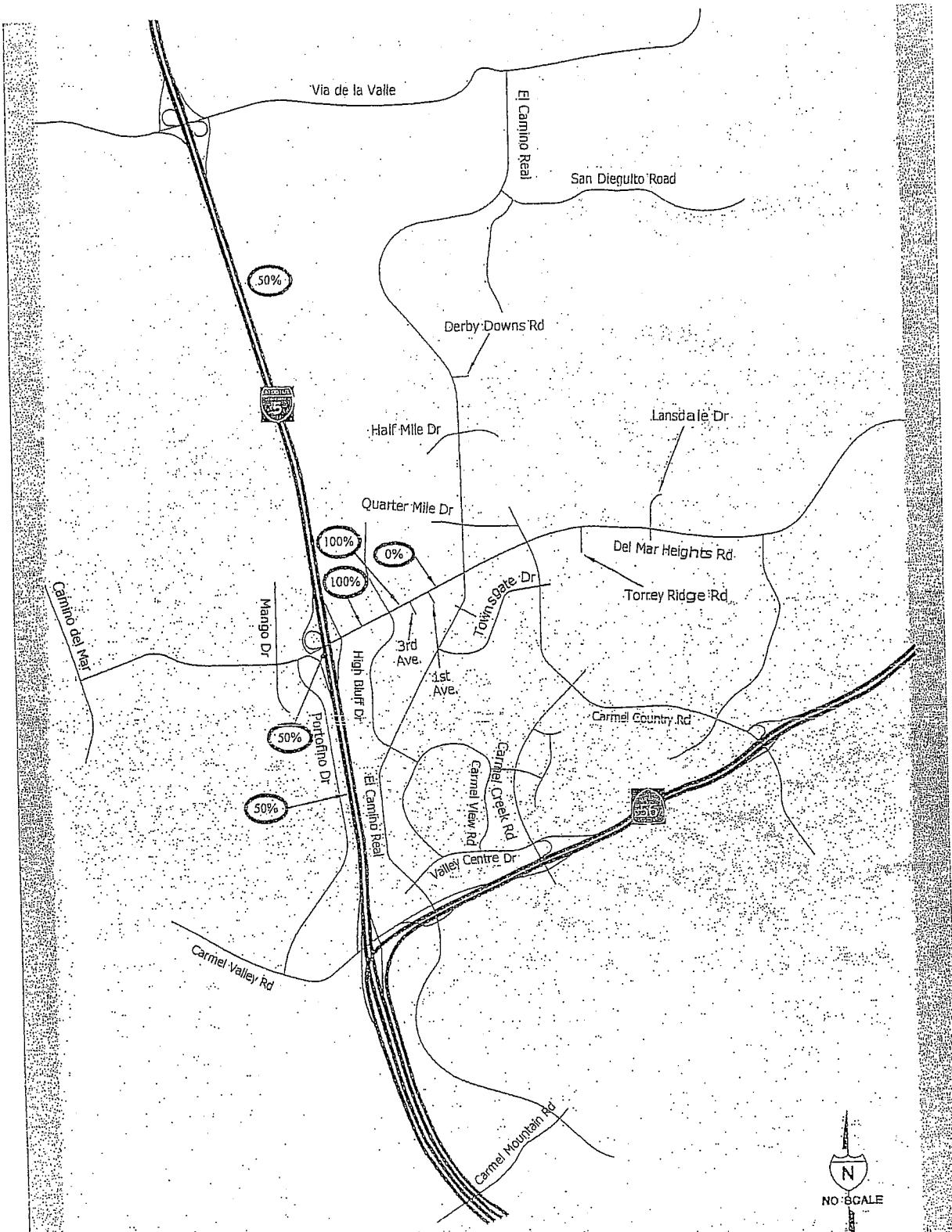
ATTACHMENT 2





ATTACHMENT 3

Employee Construction Traffic Distribution Percentages



ATTACHMENT 4
Truck & Delivery Construction Traffic Distribution Percentages

Attachment 5

One Paseo Construction Analysis

Construction Phases	Construction Duration	Project's Trip Generation ADT by phase	Percentage of ADT by phase	Assumed Percentage of Cumulative Projects
Phase 1	28 months	9,888 ADT	37%	37%
Phase 2	22 months	17,812 ADT	66%	66%
Phase 3	31 months	26,961 ADT	100%	100%
Phase 1 & 2	28 months	17,812 ADT	66%	66%
Phase 1-3	40 months	26,961 ADT	100%	100%

Source: Whiting-Turner

ATTACHMENT 6
One Paseo Trip Generation Table
Construction Traffic

PHASE 1

Purpose	Number	Auto Equivalency	Equivalent Autos	Trip	ADT	AM Peak Hour						PM Peak Hour					
						%*	#	In	: Out	In	Out	%*	#	In	: Out	In	Out
Employees	300 Autos	N/A	300	2 /Auto	600	4%	24	9	: 1	22	2	4%	24	2	: 8	5	19
Material Deliveries	25 Trucks	2.5	62.5	2 /Auto	125	9%	11	4	: 6	5	7	8%	10	5	: 5	5	5
Truck Imports/Exports	210 Trucks	2.5	525	2 /Auto	1,050	9%	95	4	: 6	38	57	8%	84	5	: 5	42	42
TOTAL							1,775			64	66		118			52	66

Notes:

Passenger-Car equivalents for Trucks is 2.5 per Exhibit 21-9 in the Highway Capacity Manual 2000

Typical Work Hours 7AM to 3:30PM.

For Employee Peak Hour In/Out Ratios, a 4% AM and PM peak is assumed based on the AM peak counts beginning at 7:30AM and the majority of employee shifts ending at 3:30PM, which is prior to the PM peak counts beginning at 5:00PM.

For Material Deliveries and Truck Imports/Exports, the Truck Terminal land use peak hour splits were used based on the City of San Diego Trip Generation Manual, May 2003.

ATTACHMENT 7

Existing Without & Existing With Construction (Phase 1)
Street Segment Comparison

Road	Segment	Class.	Existing			Existing + Construction (Phase 1)			$\Delta V/C$	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,888	0.818	0.016	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	51,625	0.860	D	53,160	0.886	0.026	NO
	High Bluff Drive to First Avenue	PA	C	37,910	0.632	C	39,445	0.657	0.026	NO
	First Avenue to El Camino Real	PA	C	37,910	0.632	C	38,150	0.636	0.004	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,734	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,955	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	14,731	0.295	A	14,881	0.298	0.003	NO

Legend:

LOS= Level of Service

V/C= Volume to Capacity Ratio

$\Delta V/C$ = Change in V/C ratio

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

ATTACHMENT 8

Existing & Existing + Construction Traffic (Phase I) Intersection Summary

#	Intersection	Existing				Existing + Construction Traffic (Phase I)			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		D	LOS	D	LOS	D	LOS	D	LOS
1	Del Mar Heights Road / I-5 SB Ramps	22.5	C	20.3	C	22.7	C	0.2	No
2	Del Mar Heights Road / I-5 NB Ramps	35.1	D	37.5	D	35.2	D	0.1	No
3	Del Mar Heights Road / High Bluff Drive	26.1	C	28.9	C	26.8	C	0.7	No
4	Del Mar Heights Road / First Avenue	DNE	DNE	DNE	DNE	3.4	A	N/A	No
5	Del Mar Heights Road / El Caninto Ranch	27.2	C	26.9	C	28.0	C	0.8	No

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D= Delay

DNE = Does Not Exist

N/A = Not Applicable

ATTACHMENT 9

Existing With & Without Construction Traffic (Phase 1)

Freeway Level of Service Summary

Segment	Dir.	Existing		Existing + Construction (Phase 1)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6447	C	0.6453	C	0.0006	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6655	C	0.6661	C	0.0006	NO
Del Mar Heights Rd./ SR-56	NB	0.5565	B	0.5570	B	0.0005	NO
Del Mar Heights Rd./ SR-56	SB	0.5744	B	0.5749	B	0.0005	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.?= Is this significant?

ATTACHMENT 10

Near Term With & Without Construction (Phase 1) Street Segment Comparison

Road	Segment	Class.	Near Term			Near Term + Construction (Phase 1)			ΔV/C	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,888	0.818	0.016	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	52,217	0.870	D	53,752	0.896	0.026	NO
	High Bluff Drive to First Avenue	PA	C	38,502	0.642	C	40,037	0.667	0.026	NO
	First Avenue to El Camino Real	PA	C	38,502	0.642	C	38,742	0.646	0.004	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,734	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,955	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	15,412	0.308	A	15,562	0.311	0.003	NO

Legend:

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

ATTACHMENT 11

Near Term With & Without Construction Traffic Intersection Summary
Phase 1

#	Intersection	Near Term						Near Term + Construction Traffic (Phase 1)					
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour		AM		S?	
		D	LOS	D	LOS	D	LOS	D	LOS	A	S?	A	S?
1	Del Mar Heights Road / I-5 SB Ramps	21	C	21.7	C	23.2	C	2.2	N	22.0	C	0.3	N
2	Del Mar Heights Road / I-5 NB Ramps	34.6	C	36.7	D	35.5	D	0.9	N	37.6	D	0.9	N
3	Del Mar Heights Road / High Bluff Drive	28.1	C	31.4	C	29	C	0.9	N	31.6	C	0.2	N
4	Del Mar Heights Road / First Avenue	DNE	DNE	DNE	DNE	3.5	A	0.0	N	4.9	A	0.0	N
5	Del Mar Heights Road / El Camino Real	27.7	C	27.9	C	28.2	C	0.5	N	28.5	C	0.6	N

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D= Delay

ATTACHMENT 12

Near Term With & Without Construction Traffic
Freeway Level of Service Summary
Phase 1

Segment	Dir.	Near Term		Near Term + Construction (Phase 1)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6460	C	0.6466	C	0.0006	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6667	C	0.6674	C	0.0006	NO
Del Mar Heights Rd./ SR-56	NB	0.5576	B	0.5581	B	0.0005	NO
Del Mar Heights Rd./ SR-56	SB	0.5755	B	0.5761	B	0.0005	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.? = Is this significant?

ATTACHMENT 13
One Paseo Trip Generation Table
Construction Traffic

PHASE 2

Purpose	Number	Auto Equivalency	Equivalent Autos	Trip	ADT	AM Peak Hour						PM Peak Hour					
						%*	#	In	Out	In	Out	%*	#	In	Out	In	Out
Employees	300 Autos	N/A	300	2 /Auto	600	4%	24	9 : 1	22	2	4%	24	2 : 8	5	19		
Material Deliveries	25 Trucks	2.5	62.5	2 /Auto	125	9%	11	4 : 6	5	7	8%	10	5 : 5	5	5		
Truck Imports/Exports	108 Trucks	2.5	270	2 /Auto	540	9%	49	4 : 6	19	29	8%	43	5 : 5	22	22		
TOTAL						1,265		84		46	38		77		31		46

Notes:

Passenger-Car equivalents for Trucks is 2.5 per Exhibit 21-9 in the Highway Capacity Manual 2000

Typical Work Hours 7AM to 3:30PM.

For Employee Peak Hour In/Out Ratios, a 4% AM and PM peak is assumed based on the AM peak counts beginning at 7:30AM and the majority of employee shifts ending at 3:30PM, which is prior to the PM peak counts beginning at 5:00PM.

For Material Deliveries and Truck Imports/Exports, the Truck Terminal land use peak hour splits were used based on the City of San Diego Trip Generation Manual, May 2003.

ATTACHMENT 14

**Existing Without & Existing With Construction (Phase 2)
Street Segment Comparison**

Road	Segment	Class.	Existing			Existing + Construction (Phase 2)			$\Delta V/C$	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,633	0.813	0.011	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	51,625	0.860	D	52,650	0.878	0.017	NO
	High Bluff Drive to First Avenue	PA	C	37,910	0.632	C	38,935	0.649	0.017	NO
	First Avenue to El Camino Real	PA	C	37,910	0.632	C	38,150	0.636	0.004	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,734	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,955	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	14,731	0.295	A	14,881	0.298	0.003	NO

Legend:

LOS= Level of Service

V/C= Volume to Capacity Ratio

$\Delta V/C$ = Change in V/C ratio

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

ATTACHMENT 15

Existing & Existing + Construction Traffic (Phase 2) Intersection Summary

#	Intersection	Existing				Existing + Construction Traffic (Phase 2)			
		AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour	Δ	S?
D	LOS	D	LOS	D	LOS	D	LOS	D	LOS
1	Del Mar Heights Road / I-5 SB Ramps	22.5	C	20.3	C	22.7	C	0.2	No
2	Del Mar Heights Road / I-5 NB Ramps	35.1	D	37.5	D	35.2	D	0.1	No
3	Del Mar Heights Road / High Bluff Drive	26.1	C	28.9	C	26.5	C	0.4	No
4	Del Mar Heights Road / Third Avenue	DNE	DNE	DNE	DNE	3	A	N/A	N/A
5	Del Mar Heights Road / El Camino Real	27.2	C	26.9	C	27.5	C	0.3	No

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D= Delay

DNE = Does Not Exist

N/A = Not Applicable

ATTACHMENT 16

Existing With & Without Construction Traffic (Phase 2)

Freeway Level of Service Summary

Segment	Dir.	Existing		Existing + Construction (Phase 2)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6447	C	0.6452	C	0.0004	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6655	C	0.6660	C	0.0004	NO
Del Mar Heights Rd./ SR-56	NB	0.5565	B	0.5568	B	0.0004	NO
Del Mar Heights Rd./ SR-56	SB	0.5744	B	0.5748	B	0.0004	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.?= Is this significant?

ATTACHMENT 17

**Near Term With & Without Construction (Phase 2)
Street Segment Comparison**

Road	Segment	Class.	Near Term			Near Term + Construction (Phase 2)			ΔV/C	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,633	0.813	0.011	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	52,682	0.878	D	53,707	0.895	0.017	NO
	High Bluff Drive to First Avenue	PA	C	38,967	0.649	C	39,992	0.667	0.017	NO
	First Avenue to El Camino Real	PA	C	38,967	0.649	C	39,207	0.653	0.004	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,734	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,955	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	15,946	0.319	A	16,096	0.322	0.003	NO

Legend:

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

LOS= Level of Service

V/C= Volume to Capacity Ratio

ΔV/C= Change in V/C ratio

ATTACHMENT 18

Near Term With & Without Construction Traffic Intersection Summary
Phase 2

#	Intersection	Near Term				Near Term + Construction Traffic (Phase 2)					
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour		Δ	S?
		D	LOS	D	LOS	D	LOS	D	LOS		
1	Del Mar Heights Road / I-5 SB Ramps	21.2	C	21.9	C	23.6	C	2.4	N	22.1	C
2	Del Mar Heights Road / I-5 NB Ramps	36.8	D	37.6	D	37.5	D	0.7	N	38.0	D
3	Del Mar Heights Road / High Bluff Drive	28.2	C	31.6	C	28.8	C	0.6	N	31.8	C
4	Del Mar Heights Road / Third Avenue	DNE	DNE	DNE	DNE	3.3	A	0.0	N	4.0	A
5	Del Mar Heights Road / El Camino Real	27.9	C	30.8	C	28.3	C	0.4	N	31.4	C

Notes:

LOS = Level of Service

 Δ = Change

S = Significant

D= Delay

DNE = Does not Exist

ATTACHMENT 19

Near Term With & Without Construction Traffic
Freeway Level of Service Summary
Phase 2

Segment	Dir.	Near Term		Near Term + Construction (Phase 2)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6469	C	0.6474	C	0.0004	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6677	C	0.6681	C	0.0004	NO
Del Mar Heights Rd./ SR-56	NB	0.5585	B	0.5589	B	0.0004	NO
Del Mar Heights Rd./ SR-56	SB	0.5764	B	0.5768	B	0.0004	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.? = Is this significant?

ATTACHMENT 20
One Paseo Trip Generation Table
Construction Traffic

PHASE 3

Purpose	Number	Auto Equivalence	Equivalent Autos	Trip	ADT	AM Peak Hour						PM Peak Hour					
						%	#	In	Out	In	Out	%	#	In	Out	In	Out
Employees	300 Autos	N/A	300	2 /Auto	600	4%	24	9	1	22	2	4%	24	2	8	5	19
Material Deliveries	25 Trucks	2.5	62.5	2 /Auto	125	9%	11	4	6	5	7	8%	10	5	5	5	5
Truck Imports/Exports	129 Trucks	2.5	322	2 /Auto	644	9%	58	4	6	23	35	8%	52	5	5	26	26
TOTAL						1,369		93		49	44		86			36	50

Notes:

Passenger-Car equivalents for Trucks is 2.5 per Exhibit 21-9 in the Highway Capacity Manual 2000

Typical Work Hours 7AM to 3:30PM.

For Employee Peak Hour In/Out Ratios, a 4% AM and PM peak is assumed based on the AM peak counts beginning at 7:30AM and the majority of employee shifts ending at 3:30PM, which is prior to the PM peak counts beginning at 5:00PM.

For Material Deliveries and Truck Imports/Exports, the Truck Terminal land use peak hour splits were used based on the City of San Diego Trip Generation Manual, May 2003.

ATTACHMENT 21

**Existing Without & Existing With Construction (Phase 3)
Street Segment Comparison**

Road	Segment	Class.	Existing			Existing + Construction (Phase 3)			ΔV/C	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,684	0.814	0.012	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	51,625	0.860	D	52,754	0.879	0.019	NO
	High Bluff Drive to First Avenue	PA	C	37,910	0.632	C	39,039	0.651	0.019	NO
	First Avenue to El Camino Real	PA	C	37,910	0.632	C	38,150	0.636	0.004	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,734	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,955	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	14,731	0.295	A	14,881	0.298	0.003	NO

Legend:

LOS= Level of Service

V/C= Volume to Capacity Ratio

ΔV/C= Change in V/C ratio

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

ATTACHMENT 22

Existing & Existing + Construction Traffic (Phase 3) Intersection Summary

#	Intersection	Existing				Existing + Construction Traffic (Phase 3)							
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour		Δ			
		D	LOS	D	LOS	D	LOS	D	LOS	D	LOS		
1	Del Mar Heights Road / I-5 SB Ramps	22.5	C	20.3	C	22.8	C	0.3	No	20.9	C	0.6	No
2	Del Mar Heights Road / I-5 NB Ramps	35.1	D	37.5	D	35.3	D	0.2	No	37.9	D	0.4	No
3	Del Mar Heights Road / High Bluff Drive	26.1	C	28.9	C	26.6	C	0.5	No	29.9	C	1.0	No
4	Del Mar Heights Road / First Avenue	DNE	DNE	DNE	DNE	3.0	A	N/A	No	4.4	A	N/A	No
5	Del Mar Heights Road / El Capino Real	27.2	C	26.9	C	27.8	C	0.6	No	27.0	C	0.1	No

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D= Delay

DNE = Does Not Exist

N/A = Not Applicable

ATTACHMENT 23

Existing With & Without Construction Traffic (Phase 3)

Freeway Level of Service Summary

Segment	Dir.	Existing		Existing + Construction (Phase 3)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6447	C	0.6452	C	0.0005	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6655	C	0.6660	C	0.0005	NO
Del Mar Heights Rd./ SR-56	NB	0.5565	B	0.5569	B	0.0004	NO
Del Mar Heights Rd./ SR-56	SB	0.5744	B	0.5748	B	0.0004	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.?= Is this significant?

ATTACHMENT 24

**Near Term With & Without Construction (Phase 3)
Street Segment Comparison**

Road	Segment	Class.	Near Term			Near Term + Construction (Phase 3)			ΔV/C	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,684	0.814	0.012	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	53,226	0.887	D	54,355	0.906	0.019	NO
	High Bluff Drive to First Avenue	PA	C	39,511	0.659	C	40,640	0.677	0.019	NO
	First Avenue to El Camino Real	PA	C	39,511	0.659	C	39,751	0.663	0.004	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,734	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,955	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	16,572	0.331	A	16,722	0.334	0.003	NO

Legend:

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

LOS= Level of Service

V/C= Volume to Capacity Ratio

ΔV/C= Change in V/C ratio

ATTACHMENT 25

**Near Term With & Without Construction Traffic Intersection Summary
Phase 3**

#	Intersection	Near Term						Near Term + Construction Traffic (Phase 3)					
		AM Peak Hour		PM Peak Hour		AM Peak Hour		S?		PM Peak Hour		Δ	
		D	LOS	D	LOS	D	LOS	D	LOS	D	LOS	S?	
1	Del Mar Heights Road / I-5 SB Ramps	21.6	C	22.1	C	23.3	C	1.7	N	22.4	C	0.3	N
2	Del Mar Heights Road / I-5 NB Ramps	40.6	D	38.6	D	41.9	D	1.3	N	39.3	D	0.7	N
3	Del Mar Heights Road / High Bluff Drive	28.4	C	32.1	C	29.0	C	0.6	N	32.5	C	0.4	N
4	Del Mar Heights Road / Third Avenue	DNE	DNE	DNE	DNE	3.2	A	0.0	N	4.5	A	0.0	N
5	Del Mar Heights Road / El Camino Real	28.4	C	36.7	D	28.9	C	0.5	N	37.7	D	1.0	N

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D = Delay

DNE = Does not Exist

ATTACHMENT 26

Near Term With & Without Construction Traffic
Freeway Level of Service Summary
Phase 3

Segment	Dir.	Near Term		Near Term + Construction (Phase 3)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6481	C	0.6485	C	0.0005	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6688	C	0.6693	C	0.0005	NO
Del Mar Heights Rd./ SR-56	NB	0.5596	B	0.5599	B	0.0004	NO
Del Mar Heights Rd./ SR-56	SB	0.5774	B	0.5779	B	0.0004	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.?= Is this significant?

ATTACHMENT 27
One Paseo Trip Generation Table
Construction Traffic

PHASES 1 & 2 Combined

Purpose	Number	Auto Equivalency	Equivalent Autos	Trip	ADT	AM Peak Hour						PM Peak Hour					
						%	#	In	:Out	In	Out	%	#	In	:Out	In	Out
Employees	400 Autos	N/A	400	2 /Auto	800	4%	32	9	: 1	29	3	4%	32	2	: 8	6	26
Material Deliveries	25 Trucks	2.5	62.5	2 /Auto	125	9%	11	4	: 6	5	7	8%	10	5	: 5	5	5
Truck Imports/Exports	210 Trucks	2.5	525	2 /Auto	1,050	9%	95	4	: 6	38	57	8%	84	5	: 5	42	42
TOTAL						1,975		138		71	67		126			53	73

Notes:

Passenger-Car equivalents for Trucks is 2.5 per Exhibit 21-9 in the Highway Capacity Manual 2000

Typical Work Hours 7AM to 3:30PM.

For Employee Peak Hour In/Out Ratios, a 4% AM and PM peak is assumed based on the AM peak counts beginning at 7:30AM and the majority of employee shifts ending at 3:30PM, which is prior to the PM peak counts beginning at 5:00PM.

For Material Deliveries and Truck Imports/Exports, the Truck Terminal land use peak hour splits were used based on the City of San Diego Trip Generation Manual, May 2003,

ATTACHMENT 28

**Existing Without & Existing With Construction (Phase 1 & 2)
Street Segment Comparison**

Road	Segment	Class.	Existing			Existing + Construction (Phase 1 & 2)			$\Delta V/C$	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,958	0.819	0.017	NO
	J-5 Northbound Ramps to High Bluff Drive	PA	D	51,625	0.860	D	53,280	0.888	0.028	NO
	High Bluff Drive to First Avenue	PA	C	37,910	0.632	C	39,565	0.659	0.028	NO
	First Avenue to El Camino Real	PA	C	37,910	0.632	C	38,230	0.637	0.005	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,754	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,965	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	14,731	0.295	A	14,931	0.299	0.004	NO

Legend:

LOS= Level of Service

V/C= Volume to Capacity Ratio

$\Delta V/C$ = Change in V/C ratio

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

ATTACHMENT 29

Existing & Existing + Construction Traffic (Phase 1 & 2) Intersection Summary

#	Intersection	Existing				Existing + Construction Traffic (Phase 1 & 2)							
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour		S?		A	
		D	LOS	D	LOS	D	LOS	D	LOS	D	LOS	D	LOS
1	Del Mar Heights Road / I-5 SB Ramps	22.5	C	20.3	C	23.4	C	0.9	No	21.0	C	0.7	No
2	Del Mar Heights Road / I-5 NB Ramps	35.1	D	37.5	D	35.9	C	0.8	No	38.2	C	0.7	No
3	Del Mar Heights Road / High Bluff Drive	26.1	C	28.9	C	26.8	C	0.7	No	30.0	C	1.1	No
4	Del Mar Heights Road / Third Avenue	DNE	DNE	DNE	DNE	4.4	A	N/A	No	6.1	A	N/A	No
5	Del Mar Heights Road / El Camino Real	27.2	C	26.9	C	27.9	C	0.7	No	27.0	C	0.1	No

Notes:

LOS = Level of Service

A = Change

S = Significant

D=Delay

DNE = Does Not Exist
N/A = Not Applicable

ATTACHMENT 30**Existing With & Without Construction Traffic (Phase 1 & 2)****Freeway Level of Service Summary**

Segment	Dir.	Existing		Existing + (Phase 1 & 2)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6447	C	0.6454	C	0.0007	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6655	C	0.6662	C	0.0007	NO
Del Mar Heights Rd./ SR-56	NB	0.5565	B	0.5570	B	0.0006	NO
Del Mar Heights Rd./ SR-56	SB	0.5744	B	0.5750	B	0.0006	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.?= Is this significant?

ATTACHMENT 31

**Near Term With & Without Construction (Phase 1 & 2)
Street Segment Comparison**

Road	Segment	Class.	Near Term			Near Term + Construction (Phase 1 & 2)			ΔV/C	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	1-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	40,958	0.819	0.017	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	52,682	0.878	D	54,337	0.906	0.028	NO
	High Bluff Drive to First Avenue	PA	C	38,967	0.649	C	40,622	0.677	0.028	NO
	First Avenue to El Camino Real	PA	C	38,967	0.649	C	39,287	0.655	0.005	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,754	0.546	0.001	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,965	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	15,946	0.319	A	16,146	0.323	0.004	NO

Legend:

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

LOS= Level of Service

V/C= Volume to Capacity Ratio

ΔV/C= Change in V/C ratio

ATTACHMENT 32

Near Term With & Without Construction Traffic Intersection Summary
Phase 1 & 2

#	Intersection	Near Term						Near Term + Constr. Traffic (Phase 1&2)					
		AM Peak Hour		PM Peak Hour		AM Peak Hour		S?		PM Peak Hour		A	
		D	LOS	D	LOS	D	LOS	D	LOS	D	LOS	A	S?
1	Del Mar Heights Road / I-5 SB Ramps	21.2	C	21.9	C	24.1	C	2.9	N	22.2	C	0.3	N
2	Del Mar Heights Road / I-5 NB Ramps	36.8	D	37.6	D	38.1	D	1.3	N	38.5	D	0.9	N
3	Del Mar Heights Road / High Bluff Drive	28.2	C	31.6	C	29.2	C	1.0	N	32.1	C	0.5	N
4	Del Mar Heights Road / Third Avenue	DNE	DNE	DNE	DNE	4.4	A	0.0	N	5.7	A	0.0	N
5	Del Mar Heights Road / El Camino Real	27.9	C	30.8	C	28.7	C	0.8	N	31.9	C	1.1	N

Notes:

LOS = Level of Service

A = Change

S = Significant

D= Delay

DNE = Does not Exist

ATTACHMENT 33

Near Term With & Without Construction Traffic
Freeway Level of Service Summary
Phase 1 & 2

Segment	Dir.	Near Term		Near Term + Construction (Phase 1&2)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6469	C	0.6476	C	0.0007	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6677	C	0.6684	C	0.0007	NO
Del Mar Heights Rd/ SR-56	NB	0.5585	B	0.5591	B	0.0006	NO
Del Mar Heights Rd/ SR-56	SB	0.5764	B	0.5770	B	0.0006	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.? = Is this significant?

ATTACHMENT 34
One Paseo Trip Generation Table
Construction Traffic

PHASES 1 - 3 Combined

Purpose	Number	Auto Equivalency	Equivalent Autos	Trip	ADT	AM Peak Hour						PM Peak Hour						
						%	#	In	:	Out	In	%	#	In	:	Out	In	Out
Employees	500 Autos	N/A	500	2 /Auto	1,000	4%	40	9	:	1	36	4	4%	40	2 :	8	8	32
Material Deliveries	25 Trucks	2.5	62.5	2 /Auto	125	9%	11	4	:	6	5	7	8%	10	5 :	5	5	5
Truck Imports/Exports	210 Trucks	2.5	525	2 /Auto	1,050	9%	95	4	:	6	38	57	8%	84	5 :	5	42	42
TOTAL						2,175		146			78	67		134			55	79

Notes:

Passenger-Car equivalents for Trucks is 2.5 per Exhibit 21-9 in the Highway Capacity Manual 2000

Typical Work Hours 7AM to 3:30PM.

For Employee Peak Hour In/Out Ratios, a 4% AM and PM peak is assumed based on the AM peak counts beginning at 7:30AM and the majority of employee shifts ending at 3:30PM, which is prior to the PM peak counts beginning at 5:00PM.

For Material Deliveries and Truck Imports/Exports, the Truck Terminal land use peak hour splits were used based on the City of San Diego Trip Generation Manual, May 2003.

ATTACHMENT 35

Existing Without & Existing With Construction (Phase 1 - 3)
Street Segment Comparison

Road	Segment	Class.	Existing			Existing + Construction (Phase 1 - 3)			Δ V/C	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	41,028	0.821	0.019	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	51,625	0.860	D	53,400	0.890	0.030	NO
	High Bluff Drive to First Avenue	PA	C	37,910	0.632	C	39,685	0.661	0.030	NO
	First Avenue to El Camino Real	PA	C	37,910	0.632	C	38,310	0.639	0.007	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,774	0.546	0.002	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,975	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	14,731	0.295	A	14,981	0.300	0.005	NO

Legend:

LOS= Level of Service

V/C= Volume to Capacity Ratio

ΔV/C= Change in V/C ratio

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

ATTACHMENT 36

Existing & Existing + Construction Traffic (Phase 1-3) Intersection Summary

#	Intersection	Existing				Existing + Construction Traffic (Phase 1-3)							
		A.M. Peak Hour		P.M. Peak Hour		A.M. Peak Hour		P.M. Peak Hour		A			
		D	LOS	D	LOS	D	LOS	D	LOS	D	LOS		
1	Del Mar Heights Road / I-5 SB Ramps	22.5	C	20.3	C	22.8	C	0.3	No	21.0	C	0.7	No
2	Del Mar Heights Road / I-5 NB Ramps	35.1	D	37.5	D	35.8	C	0.7	No	38.2	C	0.7	No
3	Del Mar Heights Road / High Bluff Drive	26.1	C	28.9	C	26.8	C	0.7	No	30.0	C	1.1	No
4	Del Mar Heights Road / First Avenue	DNE	DNE	DNE	DNE	3.4	A	N/A	No	4.9	A	N/A	No
5	Del Mar Heights Road / El Camino Real	27.2	C	26.9	C	28.8	C	1.6	No	28.2	C	1.3	No

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D= Delay

DNE = Does Not Exist
N/A = Not Applicable

ATTACHMENT 37

Existing & Without Construction Traffic (Phase 1 - 3)

Freeway Level of Service Summary

Segment	Dir.	Existing		Existing + Construction (Phase 1 - 3)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6447	C	0.6455	C	0.0007	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6655	C	0.6663	C	0.0008	NO
Del Mar Heights Rd./ SR-56	NB	0.5565	B	0.5571	B	0.0006	NO
Del Mar Heights Rd./ SR-56	SB	0.5744	B	0.5751	B	0.0006	NO

Legend:

Dir.= Direction
 V/C= Volume to Capacity Ratio
 LOS= Level of Service
 Sig.?= Is this significant?

ATTACHMENT 38

**Near Term With & Without Construction (Phase 1 - 3)
Street Segment Comparison**

Road	Segment	Class.	Near Term			Near Term + Construction (Phase 1 - 3)			$\Delta V/C$	Is this impact Significant?
			LOS	Volume	V/C	LOS	Volume	V/C		
Del Mar Heights Rd.	I-5 Southbound Ramps and I-5 Northbound Ramps	5-PA	D	40,090	0.802	D	41,028	0.821	0.019	NO
	I-5 Northbound Ramps to High Bluff Drive	PA	D	53,226	0.887	E	55,001	0.917	0.030	YES
	High Bluff Drive to First Avenue	PA	C	39,511	0.659	C	41,286	0.688	0.030	NO
	First Avenue to El Camino Real	PA	C	39,511	0.659	C	39,911	0.665	0.007	NO
	El Camino Real to Carmel Country Road	PA	B	32,674	0.545	B	32,774	0.546	0.002	NO
El Camino Real	Quarter Mile Drive to Del Mar Heights Road	4-M	A	14,925	0.373	A	14,975	0.374	0.001	NO
	Del Mar Heights Road to Townsgate Drive	6-M	A	16,572	0.331	A	16,822	0.336	0.005	NO

Legend:

PA = 6 lane Primary Arterial

5-PA = 5 lane Primary Arterial with LOS E capacity of 50,000 ADT

4-M=4 lane Major

6-M = 6 lane Major

LOS= Level of Service

V/C= Volume to Capacity Ratio

$\Delta V/C$ = Change in V/C ratio

ATTACHMENT 39

**Near Term With & Without Construction Traffic Intersection Summary
Phases 1 - 3**

#	Intersection	Near Term						Near Term + Constr. Traffic (Phase 1-3)					
		AM Peak Hour		PM Peak Hour		AM Peak Hour		S?		PM Peak Hour		A	
		D	LOS	D	LOS	D	LOS	D	LOS	D	LOS	A	S?
1	Del Mar Heights Road / I-5 SB Ramps	20.3	C	21.0	C	24.4	C	4.1	N	22.5	C	1.5	N
2	Del Mar Heights Road / I-5 NB Ramps	32.0	C	34.8	C	42.3	D	10.3	N	39.8	D	5.0	N
3	Del Mar Heights Road / High Bluff Drive	26.8	C	30.0	C	29.4	C	2.6	N	32.6	C	2.6	N
4	Del Mar Heights Road / First Avenue	DNE	DNE	DNE	DNE	3.6	A	0.0	N	5.0	A	0.0	N
5	Del Mar Heights Road / El Camino Real	26.7	C	26.0	C	29.2	C	2.5	N	28.4	C	2.4	N

Notes:

LOS = Level of Service

Δ = Change

S = Significant

D= Delay

DNE = Does not Exist

ATTACHMENT 40

Near Term With & Without Construction Traffic
Freeway Level of Service Summary
Phase 1 - 3

Segment	Dir.	Near Term		Near Term + Construction (Phase 1-3)		Δ	Sig.?
		V/C	LOS	V/C	LOS		
I-5							
Via De La Valle/Del Mar Heights Rd.	NB	0.6481	C	0.6488	C	0.0007	NO
Via De La Valle/Del Mar Heights Rd.	SB	0.6688	C	0.6696	C	0.0008	NO
Del Mar Heights Rd./ SR-56	NB	0.5596	B	0.5602	B	0.0006	NO
Del Mar Heights Rd./ SR-56	SB	0.5774	B	0.5781	B	0.0006	NO

Legend:

Dir.= Direction

V/C= Volume to Capacity Ratio

LOS= Level of Service

Sig.?= Is this significant?

HCM Signalized Intersection Capacity Analysis 1: Del Mar Heights Rd. & I-15 SB Ramps

Existing + Construction (Phase 1) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps

Movement	EBB	BBE	WB	EW	SE	SW	SR	CR
Lane Configurations	4A							
Volume (vph)	665	980	0	865	326	0	0	0
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost Time (s)	6.3	6.3	5.6	5.6	5.6	5.6	5.6	5.6
Lane Util. Factor	0.95	0.95	0.97	0.97	0.91	0.91	0.91	0.91
Fri.	1.00	1.00	0.99	0.99	0.95	0.95	0.95	0.95
Fri Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (vph)	3539	3539	3429	3429	1441	1441	1441	1441
Fri Permitted	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00
Satd. Flow (vph)	3536	3539	3429	3429	1441	1441	1441	1441
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	739	1108	0	961	362	0	0
RTOR Reduction (vph)	0	0	0	0	3	51	0	0
Lane Group Flow (vph)	0	739	1108	0	964	275	0	0
Turn Type	Perm							
Protected Phases	4	4	4	4	4	4	4	4
Permitted Phases								
Actuated Green, G(s)	46.8	46.8	21.2	21.2	27.2	27.2	27.2	27.2
Effective Green, g(s)	46.8	46.8	27.2	27.2	27.2	27.2	27.2	27.2
Actuated g/C Ratio	0.54	0.54	0.32	0.32	0.32	0.32	0.32	0.32
Clearance Time (s)			5.6	5.6	5.6	5.6	5.6	5.6
Vehicle Extension (s)			3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	1928	1928	1086	1086	456	456	456	456
vs Ratio Prot.	0.24	0.31	0.62	0.62	0.19	0.19	0.19	0.19
vs Ratio Perm			0.62	0.62	0.19	0.19	0.19	0.19
Unison Delay, d1	11.2	13.0	0.92	0.92	24.8	24.8	24.8	24.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.4	11.7	11.7	2.2	2.2	2.2	2.2
Delay (s)	1.4	1.4	40.0	40.0	27.0	27.0	27.0	27.0
Level of Service	B	B	D	D	C	C	C	C
Approach Delay (s)	11.4	13.4	36.8	36.8	15	15	15	15
Approach LOS	B	B	D	D	C	C	C	C

HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Existing + Construction (Phase 1) AM
12/9/2011
4: Del Mar Heights Road & First Ave.

Movement	TEB	TEB	TEB	TEB	NBR	NBR	NBR	NBR
Lane Configurations	1	1	1	1	1	1	1	1
Volume (vph)	100	1217	674	92	1829	59	195	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	0.91	0.97	0.95	1.00	1.00
Fri.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FII Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Sald. Flow (vph)	170	5085	1583	1770	5061	3433	3242	1770
FII Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Sald. Flow (perm)	170	5085	1583	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	120	1352	749	102	2032	66	217	14
RTO/R Reduction (vph)	0	0	378	0	4	0	12	0
Lane Group Flow (vph)	120	1352	37	102	2094	0	217	0
Turn Type	Prot							
Permitted Phases	7	4	3	6	5	2	4	6
Permitted Phases	7	4	3	6	5	2	4	6
Actuated Green, G (s)	7.9	4.3	4.3	6.7	40.1	7.0	11.1	12.9
Effective Green, g (s)	7.9	4.3	4.3	6.7	40.1	7.0	11.1	12.9
Actuated g/C Ratio	0.09	0.47	0.47	0.46	0.08	0.13	0.16	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	189	236	135	206	273	409	258	360
Vs Ratio, Prot	0.07	0.27	0.23	0.06	0.04	0.06	0.03	0.03
Vs Ratio, Perm	0.07	0.27	0.23	0.06	0.04	0.06	0.03	0.03
Cyc Ratio	0.75	0.57	0.50	0.76	0.91	0.79	0.03	0.18
Uniform Delay, d1	39.1	16.9	16.2	39.8	22.2	39.8	33.7	29.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	5.7	14.7	0.0	0.8
Delay (s)	57.3	172	167	609	273	563	33.8	345
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	19.2	19.2	19.2	29.5	36.3	36.3	36.3	36.3
Approach LOS	B	B	B	C	C	C	C	D

Intersection Summary	A
HCM Average Control Delay	26.8
HCM Volume to Capacity ratio	0.12
Actuated Cycle Length (s)	16.0
Intersection Capacity Utilization	71.0%
Analysis Period (min)	15
Critical Lane Group	c

Movement	TEB	TEB	TEB	TEB	NBR	NBR	NBR	NBR
Lane Configurations	1	1	1	1	1	1	1	1
Volume (vph)	100	1217	674	92	1829	59	195	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00	1.00	1.00
Fri.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FII Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Sald. Flow (vph)	170	5085	1583	1770	5061	3433	3242	1770
FII Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Sald. Flow (perm)	170	5085	1583	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	120	1352	749	102	2032	66	217	14
RTO/R Reduction (vph)	0	0	378	0	4	0	12	0
Lane Group Flow (vph)	120	1352	37	102	2094	0	217	0
Turn Type	Prot							
Permitted Phases	7	4	3	6	5	2	4	6
Permitted Phases	7	4	3	6	5	2	4	6
Actuated Green, G (s)	7.9	4.3	4.3	6.7	40.1	7.0	11.1	12.9
Effective Green, g (s)	7.9	4.3	4.3	6.7	40.1	7.0	11.1	12.9
Actuated g/C Ratio	0.09	0.47	0.47	0.46	0.08	0.13	0.16	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	189	236	135	206	273	409	258	360
Vs Ratio, Prot	0.07	0.27	0.23	0.06	0.04	0.06	0.03	0.03
Vs Ratio, Perm	0.07	0.27	0.23	0.06	0.04	0.06	0.03	0.03
Cyc Ratio	0.75	0.57	0.50	0.76	0.91	0.79	0.03	0.18
Uniform Delay, d1	39.1	16.9	16.2	39.8	22.2	39.8	33.7	29.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	5.7	14.7	0.0	0.8
Delay (s)	57.3	172	167	609	273	563	33.8	345
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	19.2	19.2	19.2	29.5	36.3	36.3	36.3	36.3
Approach LOS	B	B	B	C	C	C	C	D

Movement	TEB	TEB	TEB	TEB	NBR	NBR	NBR	NBR
Lane Configurations	1	1	1	1	1	1	1	1
Volume (vph)	100	1217	674	92	1829	59	195	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	0.91	1.00	1.00	1.00	1.00
Fri.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FII Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Sald. Flow (vph)	170	5085	1583	1770	5061	3433	3242	1770
FII Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Sald. Flow (perm)	170	5085	1583	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	120	1352	749	102	2032	66	217	14
RTO/R Reduction (vph)	0	0	378	0	4	0	12	0
Lane Group Flow (vph)	120	1352	37	102	2094	0	217	0
Turn Type	Prot							
Permitted Phases	7	4	3	6	5	2	4	6
Permitted Phases	7	4	3	6	5	2	4	6
Actuated Green, G (s)	7.9	4.3	4.3	6.7	40.1	7.0	11.1	12.9
Effective Green, g (s)	7.9	4.3	4.3	6.7	40.1	7.0	11.1	12.9
Actuated g/C Ratio	0.09	0.47	0.47	0.46	0.08	0.13	0.16	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	189	236	135	206	273	409	258	360
Vs Ratio, Prot	0.07	0.27	0.23	0.06	0.04	0.06	0.03	0.03
Vs Ratio, Perm	0.07	0.27	0.23	0.06	0.04	0.06	0.03	0.03
Cyc Ratio	0.75	0.57	0.50	0.76	0.91	0.79	0.03	0.18
Uniform Delay, d1	39.1	16.9	16.2	39.8	22.2	39.8	33.7	29.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	5.7	14.7	0.0	0.8
Delay (s)	57.3	172	167	609	273	563	33.8	345
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	19.2	19.2	19.2	29.5	36.3	36.3	36.3	36.3
Approach LOS	B	B	B	C	C	C	C	D

Baseline

Synchro 7 - Report
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Synchro 7 - Report
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HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Existing + Construction (Phase 1) AM
129/2011
HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Movement	EBT	EBT	WB	WB	NBT	NBT	SB	SB
Lane Configurations								
Volume (vph)	217	874	211	188	1346	92	222	98
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.91
Filt.	1.00	0.97	1.00	0.99	1.00	1.00	0.91	0.91
Filt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (prot.)	3433	4937	3433	5037	3433	5085	3433	4638
Filt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm.)	3433	4937	3433	5037	3433	5085	3433	4638
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	241	971	234	209	146	102	247	140
RTO/R Reduction (vph)	0	52	0	0	0	0	69	0
Lane Group Flow (vph)	241	1163	0	209	1588	0	247	110
Turn Type	Prot							
Protected Phases	5	3	8	5	2	1	6	4
Permitted Phases	7	4	7	4	5	2	4	4
Actuated Green (G) (s)	8.7	24.3	9.3	24.9	7.0	13.3	9.5	15.8
Effective Green (g) (s)	8.7	24.3	9.3	24.9	7.0	13.3	9.5	15.8
Actuated g/C Ratio	0.12	0.34	0.13	0.34	0.10	0.18	0.13	0.22
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	413	1657	441	1732	332	934	291	450
vs Ratio Prot.	0.07	0.23	0.06	0.32	0.07	0.01	0.05	0.12
vs Ratio Perm.	0.07	0.23	0.06	0.32	0.07	0.01	0.05	0.12
vs Ratio	0.58	0.70	0.47	0.52	0.54	0.42	0.45	0.56
Uniform Delay, d _U	30.1	20.8	29.3	22.8	31.8	24.7	25.2	25.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d _I	2.1	1.3	0.8	0.8	8.7	0.1	0.1	0.7
Delay (s)	32.2	22	30.1	20.6	40.6	24.7	24.9	24.9
Level of Service	C	C	C	D	C	C	B	C
Approach Delay (s)	23.8	15.7	30.7	33.5	26.5	26.5	31.9	31.9
Approach LOS	C	C	C	C	C	C	B	C

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	28.0	G	G
HCM Volume to Capacity ratio	0.65	C	C
Actuated Cycle Length (s)	80	C	C
Intersection Capacity Utilization	72.4	C	C
Analysis Period (min)	15	C	C
Critical Lane Group			

Movement	EBT	EBT	WB	WB	NBT	NBT	SB	SB
Lane Configurations								
Volume (vph)	217	874	211	188	1346	92	222	98
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.91
Filt.	1.00	0.97	1.00	0.99	1.00	1.00	0.91	0.91
Filt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (prot.)	3433	4937	3433	5037	3433	5085	3433	4638
Filt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm.)	3433	4937	3433	5037	3433	5085	3433	4638
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	241	971	234	209	146	102	247	140
RTO/R Reduction (vph)	0	52	0	0	0	0	69	0
Lane Group Flow (vph)	241	1163	0	209	1588	0	247	110
Turn Type	Prot							
Protected Phases	5	3	8	5	2	1	6	4
Permitted Phases	7	4	7	4	5	2	4	4
Actuated Green (G) (s)	45.7	45.7	45.7	45.7	45.7	45.7	45.7	45.7
Effective Green (g) (s)	45.7	45.7	45.7	45.7	45.7	45.7	45.7	45.7
Actuated g/C Ratio	0.53	0.53	0.53	0.53	0.53	0.53	0.53	0.53
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1872	1872	1872	1872	1872	1872	1872	1872
vs Ratio Prot.	0.28	0.36	0.28	0.36	0.28	0.36	0.28	0.36
vs Ratio Perm.	0.28	0.36	0.28	0.36	0.28	0.36	0.28	0.36
vs Ratio	0.52	0.58	0.52	0.58	0.52	0.58	0.52	0.58
Uniform Delay, d _U	15.2	14.9	15.2	14.9	15.2	14.9	15.2	14.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d _I	0.3	1.0	0.3	1.0	0.3	1.0	0.3	1.0
Delay (s)	18.8	15.9	18.8	15.9	18.8	15.9	18.8	15.9
Level of Service	B	B	B	B	B	B	B	B
Approach Delay (s)	13.1	15.6	13.1	15.6	13.1	15.6	13.1	15.6
Approach LOS	B	B	B	B	B	B	B	B

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	21.0	G	G
HCM Volume to Capacity ratio	0.75	C	C
Actuated Cycle Length (s)	119	C	C
Intersection Capacity Utilization	86.4	C	C
Analysis Period (min)	15	C	C
Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & L-15 NB Ramps

Existing + Construction (Phase 1) PM
12/29/2011
3: Del Mar Heights Road & High Bluff Drive

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SEBL	SEBR
Lane Configurations								
Volume (vph)	144	0	0	0	0	0	0	0
Ideal Flow (vphp)	1481	0	0	0	0	0	0	0
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Filt.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Filt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (prot)	3433	3639	3633	3633	3633	3633	3633	3633
Filt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	3556	3556	3556	3556	3556	3556	3556
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	201	166	0	0	0	0	0	0
RTO/R Reduction (vph)	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	261	1616	0	0	0	0	0	0
Prot.	5	32	0	0	0	0	0	0
Turn Type	Prot.	Spill	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.
Protected Phases	5	32	0	0	0	0	0	0
Permitted Phases	5	32	0	0	0	0	0	0
Actuated Green, G(s)	144	60.0	0.0	0.0	41.6	52.0	52.0	52.0
Effective Green, g(s)	144	60.0	0.0	0.0	41.6	52.0	52.0	52.0
Actuated g/C Ratio	0.12	0.50	0.35	0.43	0.48	0.48	0.48	0.48
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	412	1770	0	0	1763	549	728	649
v/s Ratio Prot.	0.08	0.07	0.23	0.20	0.35	0.04	0.32	0.04
v/s Ratio Perm.	0.05	0.04	0.15	0.12	0.26	0.02	0.03	0.02
v/C Ratio	0.85	0.83	0.22	0.21	0.57	0.71	0.90	0.46
Uniform Delay, d1	40.1	18.5	12.4	18.6	26.5	37.7	28.9	46.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.4	2.2	0.1	22.3	12	13.4	0.1	4.3
Delay (s)	62.0	21.8	12.5	70.9	27.8	51.0	20.1	46.4
Level of Service	E	C	B	E	D	C	D	D
Approach Delay (s)	16.3	24.8	28.3	45.7	24.8	28.3	45.7	24.8
Approach LOS	D	D	C	C	D	D	D	D

Intersection Summary								
HCM Average Control Delay								
HCM Volume to Capacity ratio								
Actuated Cycle Length (s)								
Intersection Capacity Utilization								
Analysis Period (min)								
c Critical Lane Group								

HCM Level of Service: D
Sum of lost time (s): 8.0
ICU Level of Service: F
Analysis Period (min): 15
c Critical Lane Group: 15

HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SEBL	SEBR
Lane Configurations								
Volume (vph)	235	1481	0	0	1040	813	615	762
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	1.00	0.95	0.91	0.95	0.95
Filt.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Filt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (prot)	3433	3639	3633	3633	3633	3633	3633	3633
Filt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	3556	3556	3556	3556	3556	3556	3556
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	201	166	0	0	1156	963	683	11
RTO/R Reduction (vph)	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	261	1616	0	0	1156	405	540	507
Prot.	5	32	0	0	0	0	0	0
Turn Type	Prot.	Spill	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.
Protected Phases	5	32	0	0	0	0	0	0
Permitted Phases	5	32	0	0	0	0	0	0
Actuated Green, G(s)	144	60.0	0.0	0.0	41.6	52.0	52.0	52.0
Effective Green, g(s)	144	60.0	0.0	0.0	41.6	52.0	52.0	52.0
Actuated g/C Ratio	0.12	0.50	0.35	0.43	0.48	0.48	0.48	0.48
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	412	1770	0	0	1763	549	728	649
v/s Ratio Prot.	0.08	0.07	0.23	0.20	0.35	0.04	0.32	0.04
v/s Ratio Perm.	0.05	0.04	0.15	0.12	0.26	0.02	0.03	0.02
v/C Ratio	0.85	0.83	0.22	0.21	0.57	0.71	0.90	0.46
Uniform Delay, d1	40.1	18.5	12.4	18.6	26.5	37.7	28.9	46.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.4	2.2	0.1	22.3	12	13.4	0.1	4.3
Delay (s)	62.0	21.8	12.5	70.9	27.8	51.0	20.1	46.4
Level of Service	E	C	B	E	D	C	D	D
Approach Delay (s)	16.3	24.8	28.3	45.7	24.8	28.3	45.7	24.8
Approach LOS	D	D	C	C	D	D	D	D

HCM Level of Service: D
Sum of lost time (s): 8.0
ICU Level of Service: F
Analysis Period (min): 15
c Critical Lane Group: 15

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.

Existing + Construction (Phase 1) PM
12/9/2011

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Existing + Construction (Phase 1) PM
12/9/2011

Movement	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB
Lane Configurations																									
Lane Configurations	A4	7	4	4	5	7	4	4	7	4	A4	7	4	4	7	4	4	7	4	4	7	4	4	7	4
Volume (vph)	2145	31	21	1189	26	26	1300	1900	1900	1900	1358	361	102	746	176	270	394	238	147	146	146	147	146	146	147
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.91	1.00	0.97	0.91	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.97	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.97	
Flt	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Flt Protected	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Said. Flow (prot)	5085	3433	5085	1770	3433	1770	3433	1770	1583	1583	3433	1770	3433	1770	1583	1583	3433	1770	3433	1770	3433	1770	3433	1770	
Flt Permitted	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Said. Flow (perm)	5085	1583	5085	1583	5085	1583	5085	1583	1583	1583	5085	1583	5085	1583	5085	1583	5085	1583	5085	1583	5085	1583	5085	1583	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	10	0	0	0	27	0	0	27	0	0	44	0	0	44	0	0	0	0	0	0	217	0	181	0
RTOR Reduction (vph)	0	2883	24	23	1321	44	23	1321	44	23	1321	44	23	1321	44	23	1321	44	23	1321	44	23	1321	44	23
Lane Group Flow (vph)	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	2883	
Turn Type	Perm	Prot																							
Projected Phases	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	
Permitted Phases	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	
Actuated Green, G (s)	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	
Effective Green, g (s)	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	38.5	
Actuated g/C Ratio	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Vehicle Extension(s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	3547	1104	37	3970	131	118	131	118	131	118	131	118	131	118	131	118	131	118	131	118	131	118	131	118	131
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	
Approach LOS	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
W/Ratio Prot	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	
W/Ratio Perm	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	
W/C Ratio	0.67	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	
Uniform Delay, d1	4.8	2.6	27.2	1.8	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	24.3	2.37	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	
Delay (s)	5.3	2.6	55.4	1.8	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	26.0	2.37	
Level of Service	A	A	E	A	A	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	C	
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	4.7	HCM Volume to Capacity ratio	27.1
HCM Volume to Capacity ratio	0.65	Actuated Cycle Length (s)	20.4
Actuated Cycle Length (s)	55.2	Sum of lost time (s)	76.8
Intersection Capacity Utilization	51.4%	ICU Level of Service	65.8%
Analysis Period (min)	15	Approach LOS	15
Critical Lane Group	C	Approach LOS	C

Baseline

Synchro 7 Report
Page 1

Synchro 7 - Report
Page 1

Movement	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB	EB	WB	NB	WB	NB
Lane Configurations																									
Lane Configurations	A4	7	4	4	5	7	4	4	7	4	A4	7	4	4	7	4	4	7	4	4	7	4	4	7	
Volume (vph)	2145	31	21	1189	26	26	1300	1900	1900	1900	1358	361	102	746	176	270	394	238	147	146	146	147	146	146	147
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.91	1.00	0.97	0.91	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.97	0.91	0.91	0.97	0.91	0.91	0.97	0.91	0.91	0.97	0.91	0.91	
Flt	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Flt Protected	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Said. Flow (prot)	5085	3433	5085	1770	3433	1770	3433	1770	1583	1583	3433	1770	3433	1770	1583	1583	3433	1770	3433	1770	3433	1770	3433	1770	
Flt Permitted	1.00	1.00	0.95	1.00																					

HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd & I-15 SB Ramps

Near Term + Const. (Phase 1) AM
3/5/2012

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	0	687	1028	0	910	336
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0
Lane Util. Factor	0.95	0.95	0.97	0.91	0.95	0.95
Filt	1.00	1.00	0.99	0.85	1.00	0.87
Filt Protected	1.00	1.00	0.95	1.00	1.00	0.95
Satl. Flow (prot)	3539	3539	3429	1441	3539	3539
Filt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satl. Flow (perm)	3539	3539	3429	1441	3539	3539
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	763	1142	0	1011	373
RTOR Reduction (vph)	0	0	0	0	3	41
Lane Group Flow (vph)	0	763	1142	0	1045	255
Turn Type				Perm		
Protected Phases	2.6	6.2	4			
Permitted Phases			4			
Actuated Green, G (s)	46.2	46.2	29.0	29.0	43.6	43.6
Effective Green, g (s)	46.2	46.2	29.0	29.0	54.1	43.6
Actuated g/C Ratio	0.53	0.53	0.33	0.33	0.57	0.45
Clearance Time (s)			5.6	5.6	4.0	4.0
Vehicle Extension (s)			3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1877	1877	1142	480	295	2017
v/s Ratio Prot	0.22	0.32	0.30		0.42	0.32
v/s Ratio Perm			0.20			0.33
vic Ratio	0.41	0.61	0.92	0.62	0.73	0.87
Uniform Delay, d1	12.2	14.2	27.9	24.4	19.0	54.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.6	11.3	2.3	2.4	23.3
Delay (s)	124.3	147.1	39.1	26.7	21.3	77.5
Level of Service	B	B	D	C	E	C
Approach Delay (s)	124.3	147.1	36.1	36.1	29.7	32.5
Approach LOS	B	B	D	C	C	C

Intersection Summary

HCM Average Control Delay	35.5	HCM Level of Service	D
HCM Volume to Capacity ratio	0.86		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	94.1%	ICU Level of Service	F
Analysis Period (min)	15	c Critical Lane Group	15

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HCM Signalized Intersection Capacity Analysis

3: Del Mar Heights Road & High Bluff Drive

Near Term + Const. (Phase 1) AM
3/5/2012

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.

Near Term + Const. (Phase 1) AM
3/5/2012

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SBR
Lane Configurations								
Volume (vph)	111	1322	694	95	1890	61	201	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.97	0.95	1.00
Ft	1.00	1.00	0.85	1.00	1.00	1.00	0.92	1.00
Ft Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5061	3433	3242	1770
Ft Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1583	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	90	123	1469	106	210	68	223	11
RTOR Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1469	396	106	264	0	223	13
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.20
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	160	2376	740	134	2290	272	411	264
Vs Ratio Prot	0.07	0.29	0.25	0.06	0.043	0.06	0.00	0.04
Vs Ratio Perm								
Vs Ratio	0.77	0.62	0.54	0.79	0.95	0.82	0.03	0.34
Uniform Delay, d1	39.3	17.6	16.7	40.2	23.1	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.5	0.7	26.4	9.0	17.3	0.0	0.8
Delay (s)	58.9	18.1	17.5	66.6	32.1	57.4	33.9	34.5
Level of Service	E	B	B	C	C	E	C	D
Approach Delay (s)	20.0	33.7	33.7	20.0	55.0	37.0	37.0	37.0
Approach LOS	C	C	D	C	D	D	D	D

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SBR
Lane Configurations								
Volume (vph)	1571		1571		38	26	2039	40
Ideal Flow (vph)	1900		1900		1900	1900	1900	1900
Total Lost time (s)	4.0		4.0		4.0	4.0	4.0	4.0
Lane Util. Factor	0.91		1.00		0.97	0.91	1.00	1.00
Ft	1.00		0.95		1.00	1.00	1.00	0.95
Ft Protected	1.00		0.95		1.00	0.95	1.00	0.95
Satd. Flow (prot)	5085		1583		3433	5085	1770	1583
Ft Permitted	1.00		1.00		0.95	1.00	0.95	1.00
Satd. Flow (perm)	5085		1583		3433	5085	1770	1583
Peak-hour factor, PHF	0.90		0.90		0.90	0.90	0.90	0.90
Adj. Flow (vph)	1746		1746		42	29	2266	44
RTOR Reduction (vph)	0		0		11	0	0	27
Lane Group Flow (vph)	1746		1746		31	29	2266	44
Turn Type								
Protected Phases	4		4		3	8	2	2
Permitted Phases								
Actuated Green, G (s)	50.6		50.6		1.9	56.5	4.1	4.1
Effective Green, g (s)	50.6		50.6		1.9	56.5	4.1	4.1
Actuated g/C Ratio	0.74		0.74		0.03	0.82	0.06	0.06
Clearance Time (s)	4.0		4.0		4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0		3.0		3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	3761		1168		95	4188	106	95
Vs Ratio Prot	0.34		0.01		0.045	0.02		
Vs Ratio Perm								
Vs Ratio	0.47		0.03		0.31	0.54	0.42	0.00
Uniform Delay, d1	3.6		24		32.7	1.9	31.1	30.4
Progression Factor	1.00		1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1		0.0		1.8	0.1	2.6	0.1
Delay (s)	37		24		34.5	21	33.7	30.4
Level of Service	A		A		A	C	C	C
Approach Delay (s)	37		25		37.0	25	32.4	32.4
Approach LOS	A		D		D	A	C	C

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SBR
Lane Configurations								
Volume (vph)	1571		1571		38	26	2039	40
Ideal Flow (vph)	1900		1900		1900	1900	1900	1900
Total Lost time (s)	4.0		4.0		4.0	4.0	4.0	4.0
Lane Util. Factor	0.91		1.00		0.97	0.91	1.00	1.00
Ft	1.00		0.95		1.00	1.00	1.00	0.95
Ft Protected	1.00		0.95		1.00	0.95	1.00	0.95
Satd. Flow (prot)	5085		1583		3433	5085	1770	1583
Ft Permitted	1.00		1.00		0.95	1.00	0.95	1.00
Satd. Flow (perm)	5085		1583		3433	5085	1770	1583
Peak-hour factor, PHF	0.90		0.90		0.90	0.90	0.90	0.90
Adj. Flow (vph)	1746		1746		42	29	2266	44
RTOR Reduction (vph)	0		0		11	0	0	27
Lane Group Flow (vph)	1746		1746		31	29	2266	44
Turn Type								
Protected Phases	4		4		3	8	2	2
Permitted Phases								
Actuated Green, G (s)	50.6		50.6		1.9	56.5	4.1	4.1
Effective Green, g (s)	50.6		50.6		1.9	56.5	4.1	4.1
Actuated g/C Ratio	0.74		0.74		0.03	0.82	0.06	0.06
Clearance Time (s)	4.0		4.0		4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0		3.0		3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	3761		1168		95	4188	106	95
Vs Ratio Prot	0.34		0.01		0.045	0.02		
Vs Ratio Perm								
Vs Ratio	0.47		0.03		0.31	0.54	0.42	0.00
Uniform Delay, d1	3.6		24		32.7	1.9	31.1	30.4
Progression Factor	1.00		1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1		0.0		1.8	0.1	2.6	0.1
Delay (s)	37		24		34.5	21	33.7	30.4
Level of Service	A		A		A	C	C	C
Approach Delay (s)	37		25		37.0	25	32.4	32.4
Approach LOS	A		D		D	A	C	C

Intersection Summary:

HCM Average Control Delay 3.5

HCM Volume to Capacity ratio 0.53

Actuated Cycle Length (s) 68.6

Intersection Capacity Utilization 49.4%

Analysis Period (min) 15

c Critical Lane Group

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HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Near Term + Construction (Phase 1) AM
3/5/2012
1: Del Mar Heights Rd. & I-15 SB Ramps

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	223	900	286	198	1386	95	236	103	79	164	302	420
Volume (vph)												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.95	0.91
Fit	1.00	0.96	1.00	0.99	1.00	0.99	1.00	0.96	0.91	1.00	0.95	0.95
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satl. Flow (prot)	3433	4901	3433	5036	3433	5085	3433	5085	3433	3433	4642	4642
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satl. Flow (perm)	3453	4901	3453	5036	3453	5085	3453	5085	3453	3453	4642	4642
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	75	0	220	1640	106	262	114	88	182	336	467
RTO/R Reduction (vph)	0	0	0	10	0	0	0	0	0	161	0	0
Lane Group Flow (vph)	248	1243	0	220	1636	0	262	114	17	182	642	0
Turn Type	Prot	Prot	Prot	Prot								
Protected Phases	7	4	3	8	5	2	2	1	6	2	6	4
Permitted Phases												
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8			
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8			
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.08	0.19	0.19	0.13	0.23			
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0			
Lane Gap Cap (vph)	326	1674	409	1842	326	972	302	481	1057			
Vs Ratio Prot	0.07	0.25	0.06	0.32	0.08	0.02	0.01	0.05	0.14			
Vs Ratio Perm										0.29	0.37	0.30
Vc Ratio	0.76	0.74	0.54	0.89	0.80	0.12	0.06	0.40	0.92dr			
Uniform Delay, d1	32.6	21.4	30.6	22.0	32.7	24.7	24.4	29.4	25.5			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	10.0	1.8	1.4	5.6	13.4	0.1	0.1	0.6	1.0			
Delay (s)	42.6	23.3	32.0	27.6	46.1	24.8	24.5	30.0	26.5			
Level of Service	D	C	C	C	D	C	C	C	C			
Approach Delay (s)	26.3	26.7	28.1	36.7	36.7	17.2	27.2	27.2	32.6			
Approach LOS	C	C	C	D	C	B	B	B	C			

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	223	900	286	198	1386	95	236	103	79	164	302	420
Volume (vph)												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.95	0.91
Fit	1.00	0.96	1.00	0.99	1.00	0.99	1.00	0.96	0.91	1.00	0.95	0.95
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satl. Flow (prot)	3433	4901	3433	5036	3433	5085	3433	5085	3433	3433	4642	4642
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satl. Flow (perm)	3453	4901	3453	5036	3453	5085	3453	5085	3453	3453	4642	4642
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	75	0	220	1640	106	262	114	88	182	336	467
RTO/R Reduction (vph)	0	0	0	10	0	0	0	0	0	161	0	0
Lane Group Flow (vph)	248	1243	0	220	1636	0	262	114	17	182	642	0
Turn Type	Prot	Prot	Prot	Prot								
Protected Phases	7	4	3	8	5	2	2	1	6	2	6	4
Permitted Phases												
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8			
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8			
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.08	0.19	0.19	0.13	0.23			
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0			
Lane Gap Cap (vph)	326	1674	409	1842	326	972	302	481	1057			
Vs Ratio Prot	0.07	0.25	0.06	0.32	0.08	0.02	0.01	0.05	0.14			
Vs Ratio Perm										0.29	0.37	0.30
Vc Ratio	0.76	0.74	0.54	0.89	0.80	0.12	0.06	0.40	0.92dr			
Uniform Delay, d1	32.6	21.4	30.6	22.0	32.7	24.7	24.4	29.4	25.5			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	10.0	1.8	1.4	5.6	13.4	0.1	0.1	0.6	1.0			
Delay (s)	42.6	23.3	32.0	27.6	46.1	24.8	24.5	30.0	26.5			
Level of Service	D	C	C	C	D	C	C	C	C			
Approach Delay (s)	26.3	26.7	28.1	36.7	36.7	17.2	27.2	27.2	32.6			
Approach LOS	C	C	C	D	C	B	B	B	C			

Intersection Summary		HCM Average Control Delay	28.2	HCM Level of Service	C
HCM Volume to Capacity ratio	0.73	Sum of lost time (s)	12.0	ICU Level of Service	C
Actualized Cycle Length (s)	73.8	Analysis Period (min)	15	70.5%	15
Intersection Capacity Utilization	70.6%	Defacto Right Lane, Recode with 1 though lane as a right lane.			
Approach Delay (s)	14.3	Critical Lane Group			
Approach LOS	B				

Baseline

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HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps

Near Term + Construction (Phase 1) PM
3: Del Mar Heights Road & High Bluff Drive

Near Term + Construction (Phase 1) PM
35/2012

Movement	EBL	EBT	EWL	WBL	WB	WB	NBL	NBT	NBT	SBL	SBT	SBT
Lane Configurations												
Volume (vph)	242	1527	0	0	1122	863	639	15	796	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	1.00	0.95	0.91	0.95	1.00	1.00	0.91	0.95	1.00
Frt	1.00	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00	1.00	1.00
Frt Protected	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00	1.00
Satl. Flow (prot)	3433	3539	0	0	5085	1683	1681	1499	1604	0	0	0
Frt Permitted	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00	1.00
Satl. Flow (perm)	3433	3539	0	0	5085	1683	1681	1499	1604	0	0	0
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1697	0	0	1247	959	710	17	884	0	0	0
RTO/R Reduction (vph)	0	0	0	0	0	523	0	6	6	0	0	0
Lane Group Flow (vph)	269	1697	0	0	1247	436	561	531	507	0	0	0
Turn Type	Prot	Prot	Split	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	5	2	6	6	8	8	8	8	8	3	4	4
Permitted Phases	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Actuated Green, G (s)	11.5	62.6	47.1	47.1	49.4	49.4	49.4	47.1	53.8	1.7	37.8	22.2
Effective Green, g (s)	11.5	62.6	47.1	47.1	49.4	49.4	49.4	47.7	53.8	1.7	37.8	22.2
Actuated g/C Ratio	0.10	0.52	0.39	0.39	0.41	0.41	0.41	0.18	0.54	0.02	0.38	0.22
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grip Cap (vph)	329	1846	1986	621	692	617	619	312	222	847	30	1907
v/s Ratio Prot	0.08	c0.48	0.25	0.28	0.33	c0.35	0.34	0.016	0.46	0.12	0.01	0.21
v/s Ratio Perm	0.32	0.32	0.62	0.70	0.81	0.86	0.82	0.89	0.85	0.23	0.57	0.76
v/c Ratio	0.32	0.24	0.293	0.306	0.312	0.322	0.313	0.404	0.20	0.24	0.48	0.274
Uniform Delay, d1	53.2	26.4	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	14.5	8.9	1.5	6.5	7.2	11.8	8.3	24.8	2.8	0.1	22.3	1.8
Delay (s)	67.7	35.3	30.8	37.1	38.3	44.0	39.7	65.2	22.8	12.5	71.3	29.3
Level of Service	E	D	C	D	D	D	D	E	C	B	C	C
Approach Delay (s)	39.7	0	33.5	0	40.6	0	0	25.8	29.7	0	50.2	0
Approach LOS	D	D	C	C	D	D	A	C	C	C	D	D

Intersection Summary			HCM Level of Service			C		
HCM Average Control Delay	37.6	HCM Level of Service	31.6					
HCM Volume to Capacity ratio	0.89		0.82					
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	100.5					
Intersection Capacity Utilization	96.5%	ICU Level of Service	78.6%					
Analysis Period (min)	15		15					
c Critical Lane Group								

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Baseline
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HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.

Near Term + Construction (Phase 1) PM
5: Del Mar Heights Road & El Camino Real

Near Term + Construction (Phase 1) PM
3/5/2012

Movement	E BT	E BT	W BL	W BL	N BL	N BL	W BR	W BR	N BR	N BR	S BT	S BT
Lane Configurations												
Volume (vph)	2272	31	21	1475	40	26	1475	40	26	1475	40	26
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.97	0.91	1.00	1.00	0.97	0.91	0.97	0.91	1.00	0.97
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.97	1.00	0.97	1.00	0.95
Frt Protected	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sal'd. Flow (prot)	5085	1583	3433	5085	1770	1583	5085	1770	1583	5085	1770	1583
Frt Permitted	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sal'd. Flow (perm)	5085	1583	3433	5085	1770	1583	5085	1770	1583	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2524	0.94	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
R/TOR Reduction (vph)	0	10	0	0	0	27	0	0	0	0	0	0
Lane Group Flow (vph)	2524	24	23	1639	44	2	1639	44	2	1639	44	2
Turn Type	Prot	Prot	Perm	Prot	Prot	Perm	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	4	4	3	8	2	2	7	4	3	8	5	2
Permitted Phases												
Actuated Green, G (s)	38.9	38.9	0.6	43.5	4.2	4.2	40.4	4.6	28.5	10.1	15.1	6.7
Effective Green, g (s)	38.9	38.9	0.6	43.5	4.2	4.2	40.4	4.6	28.5	10.1	15.1	6.7
Actuated g/C Ratio	0.70	0.70	0.01	0.78	0.08	0.08	0.20	0.49	0.06	0.34	0.12	0.18
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3551	1106	37	3771	133	119	201	191	170	419	927	288
W/R Ratio Prot	0.50	0.01	0.01	0.32	0.02	0.02	0.15	0.03	0.03	0.21	0.11	0.05
W/R Ratio Perm												
W/R Ratio	0.71	0.02	0.62	0.41	0.33	0.02	0.81	0.62	0.60	0.91	0.49	0.54
Uniform Delay, d1	5.0	2.6	27.4	2.0	24.4	23.8	17.9	38.2	22.4	35.9	30.4	31.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	0.0	28.2	0.1	1.5	0.1	4.2	2.1	5.8	23.8	0.4	2.1
Delay (s)	5.7	2.6	55.7	2.0	25.9	23.9	35.3	19.9	44.1	23.0	59.7	30.8
Level of Service	A	A	E	A	C	C	D	B	C	C	D	C
Approach Delay (s)	6.7	2.8	25.1	2.8	25.1	2.8	41.2	23.1	25.1	41.2	34.7	34.7
Approach LOS	A	A	C	A	C	C	D	C	C	D	C	C

Intersection Summary

HCM Average Control Delay	4.9	HCM Level of Service	C
HCM Volume to Capacity ratio	0.68	Sum of lost time (s)	120
Actuated Cycle Length (s)	55.7	ICU Level of Service	A
Intersection Capacity Utilization	63.9%	Analysis Period (min)	15
Approach LOS	c	Critical Lane Group	

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Movement	E BT	E BT	W BL	W BL	N BL	N BL	W BR	W BR	N BR	N BR	S BT	S BT
Lane Configurations												
Volume (vph)	2272	31	21	1475	40	26	1475	40	26	1475	40	26
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.97	0.91	1.00	1.00	0.97	0.91	0.97	0.91	1.00	0.97
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.97	1.00	0.97	1.00	0.95
Frt Protected	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sal'd. Flow (prot)	5085	1583	3433	5085	1770	1583	5085	1770	1583	5085	1770	1583
Frt Permitted	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sal'd. Flow (perm)	5085	1583	3433	5085	1770	1583	5085	1770	1583	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2524	0.94	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
R/TOR Reduction (vph)	0	10	0	0	0	27	0	0	0	0	0	0
Lane Group Flow (vph)	2524	24	23	1639	44	2	1639	44	2	1639	44	2
Turn Type	Prot	Prot	Perm	Prot	Prot	Perm	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	4	4	3	8	2	2	7	4	3	8	5	2
Permitted Phases												
Actuated Green, G (s)	38.9	38.9	0.6	43.5	4.2	4.2	40.4	4.6	28.5	10.1	15.1	6.7
Effective Green, g (s)	38.9	38.9	0.6	43.5	4.2	4.2	40.4	4.6	28.5	10.1	15.1	6.7
Actuated g/C Ratio	0.70	0.70	0.01	0.78	0.08	0.08	0.20	0.49	0.06	0.34	0.12	0.18
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3551	1106	37	3771	133	119	201	191	170	419	927	288
W/R Ratio Prot	0.50	0.01	0.01	0.32	0.02	0.02	0.15	0.03	0.03	0.21	0.11	0.05
W/R Ratio Perm												
W/R Ratio	0.71	0.02	0.62	0.41	0.33	0.02	0.81	0.62	0.60	0.91	0.49	0.54
Uniform Delay, d1	5.0	2.6	27.4	2.0	24.4	23.8	17.9	38.2	22.4	35.9	30.4	31.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	0.0	28.2	0.1	1.5	0.1	4.2	2.1	5.8	23.8	0.4	2.1
Delay (s)	5.7	2.6	55.7	2.0	25.9	23.9	35.3	19.9	44.1	23.0	59.7	30.8
Level of Service	A	A	E	A	C	C	D	B	C	C	D	C
Approach Delay (s)	6.7	2.8	25.1	2.8	25.1	2.8	41.2	23.1	25.1	41.2	34.7	34.7
Approach LOS	A	A	C	A	C	C	D	C	C	D	C	C

Movement	E BT	E BT	W BL	W BL	N BL	N BL	W BR	W BR	N BR	N BR	S BT	S BT
Lane Configurations												
Volume (vph)	2272	31	21	1475	40	26	1475	40	26	1475	40	26
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.97	0.91	1.00	1.00	0.97	0.91	0.97	0.91	1.00	0.97
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.97	1.00	0.97	1.00	0.95
Frt Protected	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sal'd. Flow (prot)	5085	1583	3433	5085	1770	1583	5085	1770	1583	5085	1770	1583
Frt Permitted	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sal'd. Flow (perm)	5085	1583	3433	5085	1770	1583	5085	1770	1583	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2524	0.94	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
R/TOR Reduction (vph)	0	10	0	0	0	27	0	0	0	0	0	0
Lane Group Flow (vph)	2524	24	23	1639	44	2	1639	44	2	1639	44	2
Turn Type	Prot	Prot	Perm	Prot	Prot	Perm	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	4	4	3	8	2	2	7	4	3	8	5	2
Permitted Phases												
Actuated Green, G (s)	38.9	38.9	0.6	43.5	4.2	4.2	40.4	4.6	28.5	10.1	15.1	6.7
Effective Green, g (s)	38.9	38.9	0.6	43.5	4.2	4.2	40.4	4.6	28.5	10.1	15.1	6.7
Actuated g/C Ratio	0.70	0.70	0.01	0.78	0.08	0.08	0.20	0.49	0.06	0.34	0.12	0.18
Clearance Time (s)	4.0	4.0	4.0	4.0								

HCM Signalized Intersection Capacity Analysis 11; Del Mar Heights Rd. & I-15 SB Ramps

Existing + Construction (Phase 2) AM
12/9/2011

Movement	EBBL	EBRT	WBBL	WBRT	SBBL	SBRT
Lane Configurations	4	4	4	4	4	4
Volume (vph)	0	694	995	0	860	326
Initial Flow (vph)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	5.6	5.6
Lane Util. Factor	0.95	0.95	0.95	0.95	0.97	0.91
Eff. Protected	1.00	1.00	0.95	0.95	0.98	0.85
Eff. Permitted	1.00	1.00	0.95	0.95	1.00	0.95
Signed Flow (vph)	3659	3539	3429	3429	3441	3441
Lane Permitted	1.00	1.00	0.95	0.95	1.00	0.95
Signed Flow (vph)	3599	3539	3429	3429	3441	3441
Peak-hour factor: PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	758	1106	0	956	362
RITOR Reduction (vph)	0	0	0	0	3	5
Lane Group Flow (vph)	0	758	1106	0	989	276
Turn Type	Perm					
Protected Phases	26	62	44	44	4	4
Permitted Phases						
Actuated Green, G, (s)	47.2	47.2	27.2	27.2	27.2	27.2
Effective Green, g, (s)	47.2	47.2	27.2	27.2	27.2	27.2
Actuated g/C Ratio	0.95	0.95	0.92	0.92	0.93	0.93
Clearance Time (s)			5.6	5.6	5.6	5.6
Vehicle Extension (s)			3.0	3.0	3.0	3.0
Lane Cap. Cap (vph)	1936	1936	1081	1081	454	454
Vis/Ratio Prot	0.92	0.93	0.92	0.92	0.90	0.91
Vis Ratio Perm			0.92	0.92	0.90	0.91
Uniform Delay, d1	11.2	12.9	20.4	20.4	25.0	25.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.4	11.8	11.8	2.3	2.3
Delay (s)	1.13	1.83	40.2	40.2	21.3	21.3
Level of Service	B	B	D	D	C	C
Approach Delay (s)	1.13	1.83	37.0	37.0	21.3	21.3
Approach LOS	B	B	D	D	C	C
Intersection Summary						
HCM Average Control Delay			22.7	22.7		
HCM Volume / Capacity ratio			0.70	0.70		
Actuated Cycle length (s)			86.3	86.3		
Intersection Capacity / Utilization			62.9%	62.9%		
Analysis Period (min)			16	16		
Critical Lane Group			119	119		

HCM Signalized Intersection Capacity Analysis 2: Del Mar Heights Road & I-15 NB Ramps

Existing + Construction (Phase 2) AM
12/9/2011

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HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive
Existing + Construction (Phase 2) AM
12/20/2011

Existing + Construction (Phase 2) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis 4: Del Mar Heights Road & Third Ave.

HCM Signalized Intersection Capacity Analysis 4: Del Mar Heights Road & Third Ave.

Movement	EBT	EBR	WB	WB
Lane Configurations	7	7	18	1951
Volume (vph)	1271	27	18	18
Ideal Flow (vphpl)	1910	1900	1900	1900
Total Lost Time (s)	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	1.00	0.91
Fit	1.00	0.95	1.00	1.00
Fit Protected	1.00	1.00	0.95	1.00
Safe Flow (prot)	5095	1563	1770	5095
Fit Permitted	1.00	1.00	0.95	1.00
Safe Flow (perm)	5095	1563	170	5095
Peak-hour Factor, PHF	0.90	0.90	0.90	0.90
Adj. Flow (vph)	1412	30	20	2168
RTR Reduction (vph)	0	0	0	0
Lane Group Flow (vph)	1412	20	20	2168
Turn Type	Perm	Permit	Permit	Permit
Protected Phases	4	3	3	8
Permitted Phases	4	3	3	8
Articulated Green, G (s)	32.1	32.1	10	37.3
Effective Green, g (s)	32.1	32.1	1.0	37.1
Articulated g/C Ratio	0.68	0.68	0.02	0.79
Clearance Time (s)	4.0	4.0	4.0	4.0
Vehicle Extension (s)	30.0	30.0	3.0	3.0
Lane Grp Cap (vph)	3458	1077	38	3987
Vs Ratio Prot	0.28	0.01	0.01	0.43
Vs Ratio Perm	0.01			
Uniform Delay, d ₁	0.47	0.02	0.63	0.51
Progression Factor	3.3	2.4	22.9	1.9
Incremental Delay, d ₂	1.00	1.00	1.00	1.00
Delay (s)	0.1	0.0	12.5	0.2
Level of Service	2.1	2.5	5.1	2.0
Approach LOS	A	A	D	A
Approach Lane Group	A	A	A	A
Intersection Summary				
HCM Average Control Delay	3.0			
HCM Volume to Capacity Ratio	0.52			
Adjusted Cycle Length (s)	47.2			
Intersection Capacity Utilization	47.7			
Analysis Period (min)	15			
Critical Lane Group				

Existing + Construction (Phase 2) AM
12/9/2011

Existing + Construction (Phase 2) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Existing + Construction (Phase 2) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis 1: Del Mar Heights Rd. & I-15 SB Ramps

Existing + Construction (Phase 2) PM
12/9/2011

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HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps

Existing + Construction (Phase 2) PM
12/9/2011

HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Existing + Construction (Phase 2) PM
12/9/2011

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SEBL	SEBR
Lane Configurations	4/4	7/7	7/7	7/7	4/4	4/4	4/4	4/4
Volume (vph)	1033	807	615	10	757	0	0	0
Ideal Flow (vph)	1474	1474	1474	1474	1474	1474	1474	1474
Total Lost time (s)	235	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fit Protected	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (prot)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit Permitted	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (perm)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
RTO/R Reduction (vph)	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	261	1638	10	0	1148	0	0	0
Lane Group Flow (vph)	261	1638	10	0	1148	0	0	0
Turn Type	Prot	Prot	Split	Prot	Prot	Prot	Prot	Prot
Protected Phases	5	2	2	6	6	6	6	6
Permitted Phases	5	2	2	6	6	6	6	6
Adjusted Green, G (s)	12.5	56.8	12.5	56.8	12.5	56.8	12.5	56.8
Effective Green, g (s)	12.5	56.8	42.3	42.3	53.2	53.2	17.3	17.3
Adjusted g/C Ratio	0.10	0.49	0.36	0.36	0.44	0.44	0.73	0.73
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap. (vph)	358	1734	1792	558	745	665	667	665
vs Ratio Prot	0.08	0.06	0.23	0.32	0.34	0.32	0.32	0.32
vs Ratio Perm	0.73	0.94	0.64	0.67	0.72	0.77	0.73	0.73
vs Ratio	0.73	0.94	0.64	0.67	0.72	0.77	0.73	0.73
Uniform Delay, d1	52.1	26.1	32.5	32.9	27.2	28.2	27.4	27.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.3	12.0	1.8	6.2	3.3	5.3	4.0	4.0
Delay (s)	59.0	41.0	34.3	39.0	30.5	33.5	31.4	31.4
Level of Service	E	D	C	D	C	C	C	C
Approach Delay (s)	7.435	0	0	36.4	0	0	0	0
Approach LOS	D	D	C	C	C	C	C	C
Intersection Summary								
HCM Average Control Delay	37.6	HCM Level of Service	D					
HCM Volume to Capacity ratio	0.78	0.86	0.88	0.87	0.88	0.87	0.88	0.87
Actualized Cycle Length (s)	120.0	Sum of lost time (s)	80					
Intersection Capacity Utilization	91.7%	Actual Level of Service	F					
Analysis Period (min)	15	c Critical Lane Group	A					

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	37.6	0.78	D
HCM Volume to Capacity ratio	0.78	0.86	D
Actualized Cycle Length (s)	120.0	Sum of lost time (s)	80
Intersection Capacity Utilization	91.7%	Actual Level of Service	F
Analysis Period (min)	15	c Critical Lane Group	A

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SEBL	SEBR
Lane Configurations	4/4	7/7	7/7	7/7	4/4	4/4	4/4	4/4
Volume (vph)	1033	807	615	10	757	0	0	0
Ideal Flow (vph)	1474	1474	1474	1474	1474	1474	1474	1474
Total Lost time (s)	235	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fit Protected	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (prot)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit Permitted	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (perm)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
RTO/R Reduction (vph)	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	261	1638	10	0	1148	0	0	0
Lane Group Flow (vph)	261	1638	10	0	1148	0	0	0
Turn Type	Prot	Prot	Split	Prot	Prot	Prot	Prot	Prot
Protected Phases	5	2	2	6	6	6	6	6
Permitted Phases	5	2	2	6	6	6	6	6
Adjusted Green, G (s)	12.5	56.8	42.3	42.3	53.2	53.2	17.3	17.3
Effective Green, g (s)	12.5	56.8	42.3	42.3	53.2	53.2	17.3	17.3
Adjusted g/C Ratio	0.10	0.49	0.36	0.36	0.44	0.44	0.73	0.73
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap. (vph)	358	1734	1792	558	745	665	667	665
vs Ratio Prot	0.08	0.06	0.23	0.32	0.34	0.32	0.32	0.32
vs Ratio Perm	0.73	0.94	0.64	0.67	0.72	0.77	0.73	0.73
vs Ratio	0.73	0.94	0.64	0.67	0.72	0.77	0.73	0.73
Uniform Delay, d1	52.1	26.1	32.5	32.9	27.2	28.2	27.4	27.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.3	12.0	1.8	6.2	3.3	5.3	4.0	4.0
Delay (s)	59.0	41.0	34.3	39.0	30.5	33.5	31.4	31.4
Level of Service	E	D	C	D	C	C	C	C
Approach Delay (s)	7.435	0	0	36.4	0	0	0	0
Approach LOS	D	D	C	C	C	C	C	C
Intersection Summary								
HCM Average Control Delay	37.6	HCM Level of Service	D					
HCM Volume to Capacity ratio	0.78	0.86	0.88	0.87	0.88	0.87	0.88	0.87
Actualized Cycle Length (s)	120.0	Sum of lost time (s)	80					
Intersection Capacity Utilization	91.7%	Actual Level of Service	F					
Analysis Period (min)	15	c Critical Lane Group	A					

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & Third Ave.

Existing + Construction (Phase 2) PM
12/9/2011

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Movement	EBL	EBR	WBL	WBR	NBL	NBR	WBT	EBT	EBR	WBT	WB	NBT	NBR	SBT	SBR
Lane Configurations															
Volume (vph)	13	189	27	18	144	144	144	144	144	144	144	144	144	144	144
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	1.00	0.91	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91
Fr.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Said. Flow (prot.)	5085	1583	3433	1583	3433	1583	3433	1583	3433	1583	3433	1583	3433	1583	3433
Flt. Permitted	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm.)	5085	1583	3433	5085	3433	1583	3433	1583	3433	1583	3433	1583	3433	1583	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Ad. Flow (vph)	2385	90	0	0	30	20	0	0	0	0	0	0	0	0	0
RTOR Reduction (vph)	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	2385	15	14	1324	30	1	26	0	0	0	0	0	0	0	0
Turn Type	Perm	Prot	Prot	Perm	Prot										
Protected Phases	4	4	4	2	2	2	2	2	2	2	2	2	2	2	2
Permitted Phases	3	3	3	2	2	2	2	2	2	2	2	2	2	2	2
Actuated Green, G(s)	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4	38.4
Effective Green, g(s)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Actuated g/C Ratio	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3670	1143	20	4110	142	65	73	213	294	1524	370	923	287	307	799
Vs Ratio Prof.	0.47	0.01	0.01	0.26	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
Vs Ratio Perm	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
Vs Ratio	0.66	0.66	0.66	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70
Uniform Delay, d1	3.9	2.1	2.62	1.3	24.7	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5
Progression Factor	0.01	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.0	0.74	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay (s)	4.3	2.1	9.7	2.1	25.7	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5
Level of Service	A	A	F	A	C	C	C	C	C	C	C	C	C	C	C
Approach Delay (s)	4.3	1.0	4.3	1.0	24	25	25	25	25	25	25	25	25	25	25
Approach LOS	A	A	A	A	C	C	C	C	C	C	C	C	C	C	C
Intersection Summary															
HCM Average Control Delay	3.9	HCM Level of Service													
HCM Volume to Capacity ratio	0.63	A													
Actualized Cycle Length (s)	120	Sum of lost time (s)													
Intersection Capacity Utilization	65.3%	ICU Level of Service													
Analysis Period (min)	15	Approach Delay (s)													
Critical Lane Group	C	Approach LOS													

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Synchro 7 - Report
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HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Near Term + Construction (Phase 2) AM
3/5/2012

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	668	1025	0	920	336	0
Ideal flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0
Lane Util. Factor	0.95	0.95	0.97	0.91	0.95	0.95
Fit	1.00	1.00	0.99	0.85	1.00	0.87
Fit Protected	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)	3539	3539	3429	1441	3539	1476
Fit Permitted	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)	3539	3539	3429	1441	3539	1476
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	764	1139	0	1022	373
R/TOR Reduction (vph)	0	0	0	0	3	41
Lane Group Flow (vph)	0	764	1139	0	1056	295
Turn Type				Perm		
Protected Phases	26	62	4	4		
Permitted Phases					5	2
Actuated Green, G (s)	46.2	46.2	29.1	29.1	67.7	53.4
Effective Green, g (s)	46.2	46.2	29.1	29.1	67.7	53.4
Actuated g/C Ratio	0.53	0.53	0.33	0.33	0.56	0.44
Clearance Time (s)			5.6	5.6	4.0	4.0
Vehicle Extension (s)			3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	1875	1875	1144	481	1997	2263
Vs Ratio Prot	0.22	0.32	0.31	0.20	0.41	0.32
Vs Ratio Perm					0.07	0.34
Vc Ratio	0.41	0.61	0.92	0.61	0.73	0.62
Uniform Delay, d1	12.3	14.2	28.0	24.3	19.4	27.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.6	12.2	2.3	23.3	2.1
Delay (s)	12.4	14.8	40.2	26.7	77.5	29.3
Level of Service	B	B	D	C	E	E
Approach Delay (s)	12.4	14.8	36.9	36.9	30.2	34.3
Approach LOS	B	B	D	D	C	D

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	668	1025	0	920	336	0
Ideal flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0
Lane Util. Factor	0.95	0.95	0.97	0.91	0.95	0.95
Fit	1.00	1.00	0.99	0.85	1.00	0.87
Fit Protected	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	3539	3539	3429	1441	3539	1476
Fit Permitted	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)	3539	3539	3429	1441	3539	1476
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	764	1139	0	1022	373
R/TOR Reduction (vph)	0	0	0	0	3	41
Lane Group Flow (vph)	0	764	1139	0	1056	295
Turn Type				Perm		
Protected Phases	26	62	4	4		
Permitted Phases					5	2
Actuated Green, G (s)	46.2	46.2	29.1	29.1	67.7	53.4
Effective Green, g (s)	46.2	46.2	29.1	29.1	67.7	53.4
Actuated g/C Ratio	0.53	0.53	0.33	0.33	0.56	0.44
Clearance Time (s)			5.6	5.6	4.0	4.0
Vehicle Extension (s)			3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	1875	1875	1144	481	1997	2263
Vs Ratio Prot	0.22	0.32	0.31	0.20	0.41	0.32
Vs Ratio Perm					0.07	0.34
Vc Ratio	0.41	0.61	0.92	0.61	0.73	0.62
Uniform Delay, d1	12.3	14.2	28.0	24.3	19.4	27.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.6	12.2	2.3	23.3	2.1
Delay (s)	12.4	14.8	40.2	26.7	77.5	29.3
Level of Service	B	B	D	C	E	E
Approach Delay (s)	12.4	14.8	36.9	36.9	30.2	34.3
Approach LOS	B	B	D	D	C	D
Lane Configuration	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	231	1348	0	0	1476	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	0.91	0.95	0.95
Fit	1.00	1.00	0.98	0.98	1.00	0.98
Fit Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	5085	1883	1681	1476	1504	1504
Fit Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	5085	1883	1681	1476	1504	1504
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	257	1494	0	0	427	43
R/TOR Reduction (vph)	0	0	0	0	430	20
Lane Group Flow (vph)	257	1494	0	0	6040	508
Turn Type				Prot	Prot	Prot
Protected Phases	5	2			6	8
Permitted Phases					6	8
Actuated Green, G (s)	10.3	67.7			53.4	44.3
Effective Green, g (s)	10.3	67.7			53.4	44.3
Actuated g/C Ratio	0.09	0.56			0.44	0.44
Clearance Time (s)	4.0	4.0			4.0	4.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0
Lane Gap Cap (vph)	295	1997			704	646
Vs Ratio Prot	0.07	0.41			0.32	0.34
Vs Ratio Perm					0.07	0.34
Vc Ratio	0.87	0.73			0.72	0.62
Uniform Delay, d1	54.2	19.4			27.3	36.7
Progression Factor	1.00	1.00			1.00	1.00
Incremental Delay, d2	23.3	24			12.5	20.0
Delay (s)	77.5	24.9			29.3	56.0
Level of Service	E	C			C	E
Approach Delay (s)	30.2				34.3	0.0
Approach LOS	C				C	A

Intersection Summary		HCM Level of Service	
HCM Average Control Delay		37.5	D
HCM Volume to Capacity ratio		0.89	
Actuated Cycle Length (s)		120.0	F
Intersection Capacity Utilization		94.6%	
Analysis Period (min)		15	
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Near Term + Construction (Phase 2) AM
3/5/2012
HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & Third Ave.

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	111	1365	694	95	1880	61	201	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	0.91	0.97	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (vph)	1770	5085	1563	1770	5061	3433	3242	1770
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1563	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1517	771	106	2089	68	223	11
R/T/R Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1517	386	106	2153	0	223	13
Turn Type	Prot							
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.15
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	160	2376	740	134	2290	272	411	284
Vs Ratio Prot	0.07	0.30	0.25	0.06	0.43	0.06	0.00	0.05
Vs Ratio Perm								
Vic Ratio	0.77	0.84	0.54	0.79	0.94	0.82	0.03	0.34
Uniform Delay, d1	39.3	17.9	16.7	40.2	23.1	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.6	0.7	28.4	8.5	17.3	0.0	0.8
Delay (s)	58.9	18.5	17.5	66.6	31.6	57.4	33.9	34.5
Level of Service	E	B	B	C	E	C	C	D
Approach Delay (s)	20.2	33.2	33.2	20.2	55.0	37.0	37.0	37.0
Approach LOS	C	C	C	D	D	D	A	C

Intersection Summary		HCM Level of Service		HCM Average Control Delay		HCM Volume to Capacity ratio		HCM Level of Service	
HCM Average Control Delay		28.8		0.85		0.55		A	
HCM Volume to Capacity ratio		0.85		48.0		0.5		B	
Actuated Cycle Length (s)		16.0		15		22.0		C	
Intersection Capacity Utilization		72.7%		49.5%		15		A	
Analysis Period (min)		15		15		15		A	
Critical Lane Group									

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	111	1365	694	95	1880	61	201	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	0.91	0.97	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (vph)	1770	5085	1563	1770	5061	3433	3242	1770
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1563	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1517	771	106	2089	68	223	11
R/T/R Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1517	386	106	2153	0	223	13
Turn Type	Prot							
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.15
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	160	2376	740	134	2290	272	411	284
Vs Ratio Prot	0.07	0.30	0.25	0.06	0.43	0.06	0.00	0.05
Vs Ratio Perm								
Vic Ratio	0.77	0.84	0.54	0.79	0.94	0.82	0.03	0.34
Uniform Delay, d1	39.3	17.9	16.7	40.2	23.1	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.6	0.7	28.4	8.5	17.3	0.0	0.8
Delay (s)	58.9	18.5	17.5	66.6	31.6	57.4	33.9	34.5
Level of Service	E	B	B	C	E	C	C	D
Approach Delay (s)	20.2	33.2	33.2	20.2	55.0	37.0	37.0	37.0
Approach LOS	C	C	C	D	D	D	A	C

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	111	1365	694	95	1880	61	201	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (vph)	1770	5085	1563	1770	5061	3433	3242	1770
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1563	1770	5061	3433	3242	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1517	771	106	2089	68	223	11
R/T/R Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1517	386	106	2153	0	223	13
Turn Type	Prot							
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.15
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	160	2376	740	134	2290	272	411	284
Vs Ratio Prot	0.07	0.30	0.25	0.06	0.43	0.06	0.00	0.05
Vs Ratio Perm								
Vic Ratio	0.77	0.84	0.54	0.79	0.94	0.82	0.03	0.34
Uniform Delay, d1	39.3	17.9	16.7	40.2	23.1	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.6	0.7	28.4	8.5	17.3	0.0	0.8
Delay (s)	58.9	18.5	17.5	66.6	31.6	57.4	33.9	34.5
Level of Service	E	B	B	C	E	C	C	D
Approach Delay (s)	20.2	33.2	33.2	20.2	55.0	37.0	37.0	37.0
Approach LOS	C	C	C	D	D	D	A	C

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Near Term + Construction (Phase 2) AM
3/5/2012

HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Near Term + Construction (Phase 2) PM
3/5/2012

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBR
Lane Configurations											
Volume (vph)	1222	334	1365	95	237	123	79	164	306	419	299
Ideal Flow (vphm)	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6	5.6
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91
Ft	1.00	0.96	1.00	0.98	1.00	1.00	0.85	1.00	0.91	1.00	0.85
Ft Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (prot)	3433	4878	3433	5036	3433	5085	1583	3433	4844	3539	3539
Ft Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (perm)	3433	4878	3433	5036	3433	5085	1583	3433	4844	3539	3539
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	247	371	224	1539	106	263	114	88	182	340	466
RTOR Reduction (vph)	0	88	0	10	0	0	0	71	0	161	0
Lane Group Flow (vph)	247	1280	0	224	1635	0	263	114	17	182	645
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6	4	6	4
Permitted Phases											
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8	45.7	45.7
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8	46.7	45.7
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.19	0.13	0.23	0.52	0.52
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6	5.6
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	326	1666	409	1842	326	972	302	451	1057	1850	1850
v/s Ratio Prot	0.07	0.26	0.07	0.32	0.08	0.02	0.01	0.05	0.14	0.29	0.37
v/s Ratio Perm											
v/c Ratio	0.76	0.77	0.56	0.88	0.81	0.12	0.06	0.40	0.92dr	0.55	0.72
Uniform Delay, d1	32.6	21.7	30.6	22.0	32.7	24.7	24.4	29.4	25.6	13.9	15.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	9.7	2.2	1.5	5.6	13.6	0.1	0.1	0.6	1.1	0.3	1.3
Delay (s)	42.2	23.9	32.1	27.6	46.3	24.8	24.5	30.0	26.6	14.3	17.2
Level of Service	D	C	C	D	C	C	C	C	C	B	D
Approach Delay (s)	26.7	C	C	C	C	C	C	C	C	14.3	17.2
Approach LOS										B	C
Intersection Summary											
HCM Average Control Delay	28.3									22.1	C
HCM Volume to Capacity ratio	0.73									0.78	
Actuated Cycle Length (s)	73.8									11.9	
Intersection Capacity Utilization	70.6%									87.4	
Analysis Period (min)	15									70.5%	
dr: Defacto Right Lane. Record with 1 though lane as a right lane.										15	
c: Critical Lane Group											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBR
Lane Configurations											
Volume (vph)	1222	334	1365	95	237	123	79	164	306	419	299
Ideal Flow (vphm)	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6	5.6
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91
Ft	1.00	0.96	1.00	0.98	1.00	1.00	0.85	1.00	0.91	1.00	0.85
Ft Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (prot)	3433	4878	3433	5036	3433	5085	1583	3433	4844	3539	3539
Ft Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (perm)	3433	4878	3433	5036	3433	5085	1583	3433	4844	3539	3539
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	247	371	224	1539	106	263	114	88	182	340	466
RTOR Reduction (vph)	0	88	0	10	0	0	0	71	0	161	0
Lane Group Flow (vph)	247	1280	0	224	1635	0	263	114	17	182	645
Turn Type	Prot	Prot	Prot								
Protected Phases	7	4	3	8	5	2	1	6	4	6	4
Permitted Phases											
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8	45.7	45.7
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8	46.7	45.7
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.19	0.13	0.23	0.52	0.52
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6	5.6
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	326	1666	409	1842	326	972	302	451	1057	1850	1850
v/s Ratio Prot	0.07	0.26	0.07	0.32	0.08	0.02	0.01	0.05	0.14	0.29	0.37
v/s Ratio Perm											
v/c Ratio	0.76	0.77	0.56	0.88	0.81	0.12	0.06	0.40	0.92dr	0.55	0.72
Uniform Delay, d1	32.6	21.7	30.6	22.0	32.7	24.7	24.4	29.4	25.6	13.9	15.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	9.7	2.2	1.5	5.6	13.6	0.1	0.1	0.6	1.1	0.3	1.3
Delay (s)	42.2	23.9	32.1	27.6	46.3	24.8	24.5	30.0	26.6	14.3	17.2
Level of Service	D	C	C	D	C	C	C	C	C	B	D
Approach Delay (s)	26.7	C	C	C	C	C	C	C	C	14.3	17.2
Approach LOS										B	C
Lane Configurations											
Volume (vph)	1222	334	1365	95	237	123	79	164	306	419	299
Ideal Flow (vphm)	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6	5.6
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91
Ft	1.00	0.96	1.00	0.98	1.00	1.00	0.85	1.00	0.91	1.00	0.85
Ft Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (prot)	3433	4878	3433	5036	3433	5085	1583	3433	4844	3539	3539
Ft Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (perm)	3433	4878	3433	5036	3433	5085	1583	3433	4844	3539	3539
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	247	371	224	1539	106	263	114	88	182	340	466
RTOR Reduction (vph)	0	88	0	10	0	0	0	71	0	161	0
Lane Group Flow (vph)	247	1280	0	224	1635	0	263	114	17	182	645
Turn Type	Prot	Prot	Prot								
Protected Phases	7	4	3	8	5	2	1	6	4	6	4
Permitted Phases											
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8	45.7	45.7
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	14.1	14.1	9.7	16.8	46.7	45.7
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.19	0.13	0.23	0.52	0.52
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6	5.6
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	326	1666	409	1842	326	972	302	451	1057	1850	1850
v/s Ratio Prot	0.07	0.26	0.07	0.32	0.08	0.02	0.01	0.05	0.14	0.29	0.37
v/s Ratio Perm											
v/c Ratio	0.76	0.77	0.56	0.88	0.81	0.12	0.06	0.40	0.92dr	0.55	0.72
Uniform Delay, d1	32.6	21.7	30.6	22.0	32.7	24.7	24.4	29.4	25.6	13.9	15.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	9.7	2.2	1.5	5.6	13.6	0.1	0.1	0.6	1.1	0.3	1.3
Delay (s)	42.2	23.9	32.1	27.6	46.3	24.8	24.5	30.0	26.6	14.3	17.2
Level of Service	D	C	C	D	C	C	C	C	C	B	D
Approach Delay (s)	26.7	C	C	C	C						

HCM Signalized Intersection Capacity Analysis
Near Term + Construction (Phase 2) PM
3: Del Mar Heights Road & High Bluff Drive
3/5/2012

HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps
3/5/2012

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBR
Lane Configurations	1523	0	0	1156	877	644	20	800	0	0	0
Volume (vph)	242	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vph)	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.95	0.91	0.95	0.91	1.00	1.00	0.91	0.95	1.00
Fit	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00	0.90	1.00
FIT Protected	0.95	1.00	0.95	0.99	1.00	0.95	1.00	1.00	0.95	1.00	0.95
Said. Flow (prot)	3433	3539	0	5085	1583	1681	1504	1504	1504	1770	1863
FIT Permitted	0.95	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00	1.00
Said. Flow (perm)	3433	3539	0	5085	1583	1681	1504	1504	1504	1770	1863
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1692	0	0	1284	974	716	22	889	0	0
R/TOR Reduction (vph)	0	0	0	0	514	0	6	6	0	0	0
Lane Group Flow (vph)	269	1692	0	0	1284	460	566	530	519	0	0
Turn Type	Prot	Prot	Prot	Spill	Prot						
Protected Phases	5	2	6	6	8	8	8	8	7	4	5
Permitted Phases	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Actuated Green, G (s)	11.4	62.6	0	47.2	47.2	49.4	49.4	49.4	47.2	38.0	25.1
Effective Green, g (s)	11.4	62.6	0	47.2	47.2	49.4	49.4	49.4	47.2	38.0	22.2
Actuated g/C Ratio	0.10	0.52	0.38	0.39	0.41	0.41	0.41	0.41	0.40	0.54	0.54
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	326	1846	0	2000	623	692	618	619	311	2727	849
v/s Ratio Prot	0.08	cd48	0.25	0.29	0.34	0.35	0.36	0.36	0.16	0.46	0.12
v/s Ratio Perm	0.83	0.92	0.64	0.74	0.82	0.86	0.84	0.84	0.88	0.85	0.57
v/s Ratio	0.83	0.92	0.64	0.74	0.82	0.86	0.84	0.84	0.88	0.85	0.57
Uniform Delay, d1	53.3	26.3	29.5	31.1	31.3	32.1	31.7	31.7	40.6	19.9	12.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	15.5	8.7	1.6	7.6	7.5	11.3	9.7	9.7	25.6	25.8	0.1
Delay (s)	68.8	35.0	31.1	38.7	38.8	43.4	41.4	41.4	66.1	22.7	12.5
Level of Service	E	D	C	D	D	D	D	D	E	B	E
Approach Delay (s)	39.6	34.4	34.4	41.2	41.2	41.2	41.2	41.2	29.9	30.3	30.3
Approach LOS	D	C	C	D	D	D	D	D	C	C	C

Intersection Summary											
HCM Average Control Delay	38.0	HCM Level of Service	D	HCM Volume to Capacity ratio	0.88	Sum of lost time (s)	8.0	HCM Average Control Delay	31.8	HCM Level of Service	C
Actuated Cycle Length (s)	120.0	ICU Level of Service	F	Actuated Cycle Length (s)	100.7	Sum of lost time (s)	12.0	Actuated Cycle Length (s)	0.82	Sum of lost time (s)	C
Intersection Capacity Utilization	97.7%	Analysis Period (min)	15	Intersection Capacity Utilization	78.6%	Analysis Period (min)	15	Intersection Capacity Utilization	0.04	Analysis Period (min)	D
c Critical Lane Group				c Critical Lane Group				c Critical Lane Group			D

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBR
Lane Configurations	1523	0	0	1156	877	644	20	800	0	0	0
Volume (vph)	242	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vph)	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.95	0.91	0.95	0.91	1.00	1.00	0.91	0.95	1.00
Fit	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00	0.90	1.00
FIT Protected	0.95	1.00	0.95	0.99	1.00	0.95	1.00	1.00	0.95	1.00	0.95
Said. Flow (prot)	3433	3539	0	5085	1583	1681	1504	1504	1504	1770	1863
FIT Permitted	0.95	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00	1.00
Said. Flow (perm)	3433	3539	0	5085	1583	1681	1504	1504	1504	1770	1863
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1692	0	0	1284	974	716	22	889	0	0
R/TOR Reduction (vph)	0	0	0	0	514	0	6	6	0	0	0
Lane Group Flow (vph)	269	1692	0	0	1284	460	566	530	519	0	0
Turn Type	Prot	Prot	Prot	Spill	Prot						
Protected Phases	5	2	6	6	8	8	8	8	7	4	5
Permitted Phases	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Actuated Green, G (s)	11.4	62.6	0	47.2	47.2	49.4	49.4	49.4	47.2	38.0	25.1
Effective Green, g (s)	11.4	62.6	0	47.2	47.2	49.4	49.4	49.4	47.2	38.0	22.2
Actuated g/C Ratio	0.10	0.52	0.38	0.39	0.41	0.41	0.41	0.41	0.18	0.54	0.02
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	326	1846	0	2000	623	692	618	619	311	2727	849
v/s Ratio Prot	0.08	cd48	0.25	0.29	0.34	0.35	0.36	0.36	0.16	0.46	0.12
v/s Ratio Perm	0.83	0.92	0.64	0.74	0.82	0.86	0.84	0.84	0.88	0.85	0.57
v/s Ratio	0.83	0.92	0.64	0.74	0.82	0.86	0.84	0.84	0.88	0.85	0.57
Uniform Delay, d1	53.3	26.3	29.5	31.1	31.3	32.1	31.7	31.7	40.6	19.9	12.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	15.5	8.7	1.6	7.6	7.5	11.3	9.7	9.7	25.6	25.8	0.1
Delay (s)	68.8	35.0	31.1	38.7	38.8	43.4	41.4	41.4	66.1	22.7	12.5
Level of Service	E	D	C	D	D	D	D	D	E	B	E
Approach Delay (s)	39.6	34.4	34.4	41.2	41.2	41.2	41.2	41.2	29.9	30.3	30.3
Approach LOS	D	C	C	D	D	D	D	D	C	C	C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBR
Lane Configurations	1523	0	0	1156	877	644	20	800	0	0	0
Volume (vph)	242	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vph)	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.95	0.91	0.95	0.91	1.00	1.00	0.91	0.95	1.00
Fit	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00	0.90	1.00
FIT Protected	0.95	1.00	0.95	0.99	1.00	0.95	1.00	1.00	0.95	1.00	0.95
Said. Flow (prot)	3433	3539	0	5085	1583	1681	1504	1504	1504	1770	1863
FIT Permitted	0.95	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00	1.00
Said. Flow (perm)	3433	3539	0	5085	1583	1681	1504	1504	1504	1770	1863
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1692	0	0	1284	974	716	22	889	0	0
R/TOR Reduction (vph)	0	0	0	0	514	0	6	6	0	0	0
Lane Group Flow (vph)	269	1692	0	0	1284	460	566	530	519	0	0
Turn Type	Prot	Prot	Prot	Spill	Prot						
Protected Phases	5	2	6	6	8	8	8	8	7	4	5
Permitted Phases	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Actuated Green, G (s)	11.4	62.6	0	47.2	47.2	49.4	49.4	49.4	47.2	38.0	25.1
Effective Green, g (s)	11.4	62.6	0	47.2	47.2	49.4	49.4	49.4	47.2	38.0	22.2
Actuated g/C Ratio	0.10	0.52	0.38	0.39	0.41	0.41	0.41	0.41	0.18	0.54	0.02
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	326	1846	0	2000	623	692	618	619	311	2727	849
v/s Ratio Prot	0.08	cd48	0.25	0.29	0.34	0.35	0.36	0.36	0.16	0.46	0.12
v/s Ratio Perm	0.83	0.92	0.64	0.74	0.82	0.86	0.84	0.84	0.88	0.85	0.57
v/s Ratio	0.83	0.9									

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & Third Ave.

Near Term + Construction (Phase 2) PM
5: Del Mar Heights Road & El Camino Real
3/5/2012

Movement	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	2285	19	13	1527	27	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.91	0.97	1.00	0.97
Frt	1.00	0.85	1.00	1.00	0.85	1.00
Frt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	5085	1583	1770	5085	3433	1583
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	5085	1583	1770	5085	3433	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2539	21	14	1697	30	20
RTOR Reduction (vph)	0	6	0	0	19	0
Lane Group Flow (vph)	2539	15	14	1697	30	1
Turn Type	Perm	Prot	Prot	Perm	Prot	Prot
Protected Phases	4	3	8	2	4	3
Permitted Phases	4	2	2	2	3	8
Actuated Green, G (s)	38.9	38.9	0.6	43.5	2.3	2.3
Effective Green, g (s)	38.9	38.9	0.6	43.5	2.3	2.3
Actuated g/C Ratio	0.72	0.72	0.01	0.81	0.04	0.04
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	3677	1145	20	4111	147	68
vs Ratio - Prot	0.50	0.01	0.33	0.01	0.03	0.01
vs Ratio - Perm	0.01	0.70	0.44	0.20	0.01	0.00
Vc Ratio	0.69	0.01	2.1	26.5	1.5	24.9
Uniform Delay, d1	4.1	2.1	1.00	1.00	1.00	1.00
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.6	0.0	73.4	0.1	0.7	0.1
Delay (s)	4.7	2.1	100.0	1.5	25.6	24.7
Level of Service	A	A	F	A	C	C
Approach LOS	A	A	A	A	C	C

Movement	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	454	1397	395	107	766	181
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91
Frt	1.00	0.97	1.00	0.97	1.00	0.97
Frt Protected	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3433	4917	3433	4940	3433	4940
Frt Permitted	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3433	4917	3433	4940	3433	4940
Peak-hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	504	1562	439	119	851	201
RTOR Reduction (vph)	0	52	0	41	0	0
Lane Group Flow (vph)	504	1399	0	119	1011	0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	3	8	5
Permitted Phases	7	4	3	3	8	5
Actuated Green, G (s)	16.5	40.4	4.6	28.5	10.1	15.2
Effective Green, g (s)	16.5	40.4	4.6	28.5	10.1	15.2
Actuated g/C Ratio	0.20	0.49	0.06	0.34	0.12	0.18
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	683	2396	190	1688	413	932
vs Ratio - Prot	-0.15	-0.39	-0.03	-0.20	-0.13	-0.09
vs Ratio - Perm	-0.01	-0.70	-0.44	-0.20	-0.01	-0.00
Vc Ratio	0.74	0.81	0.63	0.60	1.04	0.50
Uniform Delay, d1	31.2	18.0	38.3	22.4	36.4	30.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	4.2	2.1	6.3	6.3	54.3	0.4
Delay (s)	35.3	20.1	44.6	23.0	90.7	30.8
Level of Service	D	C	F	C	D	C
Approach Delay (s)	23.2	25.2	25.2	25.2	53.4	34.7
Approach LOS	C	C	C	C	D	C
Intersection Summary						
HCM Average Control Delay	4.0	HCM Level of Service	A			
HCM Volume to Capacity ratio	0.67					
Actuated Cycle Length (s)	12.0					
Intersection Capacity Utilization	54.1 %	ICU Level of Service	A			
Analysis Period (min)	15					
c Critical Lane Group						

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HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Existing + Construction (Phase 3) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps

Existing + Construction (Phase 3) AM
12/9/2011

Movement	EEC	EBT	WBET	SBET	EEB	EBE	WBE	SBE	WB	SB	EB	EWB	SWB	EBR	WBR	NSBR	NEBR	SEBR	NEB	SWR
Lane Configurations																				
Lane Configuration	A/A	A/A	N/N																	
Volume (vph)	0	664	995	0	326	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util Factor	0.95	0.95	0.97	0.97	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
FIT	1.00	1.00	0.98	0.98	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
FIT Protected	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Satd. Flow (prot)	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	
FIT Permitted	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Satd. Flow (perm)	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	3539	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	738	1106	0	357	362	362	362	362	362	362	362	362	362	362	362	362	362	362	
RTO/R Reduction (vph)	0	0	0	0	0	3	51	51	51	51	51	51	51	51	51	51	51	51	51	
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Turn Type	Perm																			
Protected Phases	6/2	6/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	4/2	
Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	Permitted Phases	
Actuated Green, G(s)	47.2	47.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	
Effective Green, g(s)	47.2	47.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	27.2	
Actuated g/C Ratio	0.55	0.55	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	0.32	
Clearance Time (s)	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1936	1936	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	1081	
Vs Ratio/Freq	0.20	0.20	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	
Vs Ratio/Freq	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	0.19	
Vc Ratio	0.38	0.38	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	0.57	
Uniform Delay, d1	11.2	12.9	28.5	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	6.4	2.6	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Delay (s)	58.5	23.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	33.3	
Level of Service	E	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	C	D	
Approach Delay (s)	11.3	13.3	10.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	27.3	
Approach LOS	B	B	B	B	B	B	B	B	B	B	B	B	B	B	B	B	B	B	B	

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	22.8	C	
HCM Volume to Capacity ratio	0.10		
Actualized Cycle Length (s)	86.3	11.9	
Intersection Capacity Utilization (%)	62.9%	B	
Analysis Period (min)	16		
Critical Lane Group			

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	36.3	C	
HCM Volume to Capacity ratio	0.04		
Actualized Cycle Length (s)	120.0		
Intersection Capacity Utilization (%)	90.2%		
Analysis Period (min)	15		
Critical Lane Group			

Baseline

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HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Existing + Construction (Phase 3) AM
12/8/2011
4: Del Mar Heights Road & First Ave.

Movement	EBR	WB	NBR	SB	SWB	SEB	SWB	SEB
Lane Configurations								
Volume (vph)	108	1209	674	92	1815	59	114	77
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.97	0.95	1.00
Eff. Util. Factor	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00
Eff. Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5086	1583	770	5086	3433	3433	1863
Eff. Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1583	1770	5086	3433	3433	1863
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	120	1343	749	102	2017	66	217	111
RTR Reduction (vph)	0	0	378	0	4	0	12	0
Lane Group Flow (vph)	120	1343	749	102	2079	66	217	111
Prot								
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases	7	4	3	8	5	2	1	6
Actuated Green, G(s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	17.0
Effective Green, g(s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	17.0
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.46	0.08	0.19	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	150	2386	743	135	2306	273	409	259
Vs Ratio (prot)	0.01	0.28	0.23	0.06	0.44	0.06	0.03	0.13
Vs Ratio (perm)	0.75	0.56	0.50	0.76	0.90	0.79	0.03	0.34
Yc Ratio	39.1	16.8	16.2	39.8	22.1	39.8	33.7	29.6
Uniform Delay, d1	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	6.4	14.7	0.0	5.0
Delay (s)	57.3	17.1	16.7	80.9	27.5	54.5	33.8	31.5
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	192	129	129	129	129	129	129	129
Approach LOS	B	C	D	C	D	C	D	C
Intersections Summary								
HCM Average Control Delay	26.6	HCM Level of Service	C					
HCM Volume to Capacity ratio	0.82							
Adjusted Cycle Length (s)	88.0	Sum of lost time (s)	16.0					
Intersection Capacity Utilization	70.7%	ICU Level of Service	C					
Analysis Period (min)	15							
c Critical Lane Group								

Movement	EBR	WB	NBR	SB	SWB	SEB	SWB	SEB
Lane Configurations								
Volume (vph)	108	1209	674	92	1815	59	114	77
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00
Eff. Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Eff. Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5086	1583	770	5086	3433	3433	1863
Eff. Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1583	1770	5086	3433	3433	1863
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Avg. Flow (vph)	120	1343	749	102	2017	66	217	111
RTR Reduction (vph)	0	0	378	0	4	0	12	0
Lane Group Flow (vph)	120	1343	749	102	2079	66	217	111
Prot								
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases	7	4	3	8	5	2	1	6
Actuated Green, G(s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	17.0
Effective Green, g(s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	17.0
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.46	0.08	0.19	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	150	2386	743	135	2306	273	409	259
Vs Ratio (prot)	0.01	0.28	0.23	0.06	0.44	0.06	0.03	0.13
Vs Ratio (perm)	0.75	0.56	0.50	0.76	0.90	0.79	0.03	0.34
Yc Ratio	39.1	16.8	16.2	39.8	22.1	39.8	33.7	29.6
Uniform Delay, d1	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	6.4	14.7	0.0	5.0
Delay (s)	57.3	17.1	16.7	80.9	27.5	54.5	33.8	31.5
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	192	129	129	129	129	129	129	129
Approach LOS	B	C	D	C	D	C	D	C
Intersections Summary								
HCM Average Control Delay	26.6	HCM Level of Service	C					
HCM Volume to Capacity ratio	0.82							
Adjusted Cycle Length (s)	88.0	Sum of lost time (s)	16.0					
Intersection Capacity Utilization	70.7%	ICU Level of Service	C					
Analysis Period (min)	15							
c Critical Lane Group								

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HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real
12/20/2011

Existing + Construction (Phase 3) AM
1: Del Mar Heights Rd. & I-15 SB Ramps

Movement	EBI	EBB	WB	WB	NBR	NBR	SEB	SEB	
Lane Configurations									
Volume (vph)	216	871	206	186	1345	92	218	99	
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (proj)	3433	4899	3433	3433	5095	5153	3433	4638	
Frt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	3433	4899	3433	3433	5095	5153	3433	4638	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	240	968	229	206	1894	102	222	110	
RTO/R Reduction (vph)	0	50	0	0	10	0	67	0	
Lane Group Flow (vph)	240	1147	209	1886	102	242	110	177	
Turn Type	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	
Protected Phases	7	4	3	2	5	2	3	2	
Permitted Phases	4	4	4	4	4	4	4	4	
Actuated Green, G (s)	7.0	23.4	9.4	25.8	8.7	16.8	14.8	16.7	
Effective Green, g (s)	7.0	23.4	9.4	25.8	8.7	14.8	9.6	16.7	
Actuated g/C Ratio	0.10	0.32	0.13	0.35	0.12	0.20	0.13	0.21	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	328	1579	441	1775	408	1028	320	450	
Vs Ratio Prot.	0.07	0.23	0.06	0.03	0.07	0.02	0.01	0.05	
Vs Ratio Perm	0.28	0.36	0.15	0.13	0.15	0.13	0.15	0.18	
Vc Ratio	0.73	0.79	0.47	0.89	0.56	0.11	0.05	0.08	
Uniform Delay, d1	32.2	22.1	29.6	22.4	30.6	23.8	23.5	25.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	8.2	17	0.8	6.2	23	0	0.1	0.6	
Delay (s)	40.3	23.8	30.4	28.6	32.9	23.6	28	26.8	
Level of Service	D	C	C	C	C	C	C	C	
Approach Delay(s)	265	265	265	265	288	272	272	272	
Approach LOS	C	C	C	C	C	C	C	C	
Intersection Summary									
HCM Average Control Delay	27.8	HCM Level of Service		C		C		C	
HCM Volume to Capacity ratio	0.65								
Actuated Cycle Length (s)	732			Sum of lost time (s)		119		119	
Intersection Capacity Utilization	68.5%			ICU Level of Service		C		C	
Analysis Period (min)	15			Intersection Capacity Utilization		68.4%		68.4%	
c. Detacto Right Lane, Recode with 1 though lane as a right lane				Analysis Period (min)		15		15	
c Critical Lane Group				c Critical Lane Group					

Movement	EBI	EBB	WB	WB	NBR	NBR	SEB	SEB	
Lane Configurations									
Volume (vph)	216	871	206	186	1345	92	218	99	
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (proj)	3433	4899	3433	3433	5095	5153	3433	4638	
Frt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	3433	4899	3433	3433	5095	5153	3433	4638	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	240	968	229	206	1894	102	222	110	
RTO/R Reduction (vph)	0	50	0	0	10	0	67	0	
Lane Group Flow (vph)	240	1147	209	1886	102	242	110	177	
Turn Type	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	Prot.	
Protected Phases	4	4	4	4	4	4	4	4	
Permitted Phases	4	4	4	4	4	4	4	4	
Actuated Green, G (s)	46.7	145.7	46.7	145.7	46.7	145.7	46.7	145.7	
Effective Green, g (s)	46.7	145.7	46.7	145.7	46.7	145.7	46.7	145.7	
Actuated g/C Ratio	0.55	0.55	0.55	0.55	0.55	0.55	0.55	0.55	
Clearance Time (s)	5.6	5.6	5.6	5.6	5.6	5.6	5.6	5.6	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1874	1874	1874	1874	1874	1874	1874	1874	
Vs Ratio Prot.	0.28	0.36	0.15	0.13	0.15	0.13	0.15	0.18	
Vs Ratio Perm	0.52	0.67	0.52	0.67	0.52	0.67	0.52	0.67	
Vc Ratio	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	
Uniform Delay, d1	13.2	14.9	13.2	14.9	13.2	14.9	13.2	14.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.3	1.0	0.3	1.0	0.3	1.0	0.3	1.0	
Delay (s)	13.5	15.8	13.5	15.8	13.5	15.8	13.5	15.8	
Level of Service	B	B	B	B	B	B	B	B	
Approach Delay(s)	3.5	5.8	3.5	5.8	3.5	5.8	3.5	5.8	
Approach LOS	B	B	B	B	B	B	B	B	
Intersection Summary									
HCM Average Control Delay	20.9	HCM Level of Service		C		C		C	
HCM Volume to Capacity ratio	0.65								
Actuated Cycle Length (s)	732			Sum of lost time (s)		119		119	
Intersection Capacity Utilization	68.3%			ICU Level of Service		C		C	
Analysis Period (min)	15			Intersection Capacity Utilization		68.4%		68.4%	
c Critical Lane Group				c Critical Lane Group					

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HCM Signalized Intersection Capacity Analysis
2; Del Mar Heights Road & I-15 NB Ramps

Existing + Construction (Phase 3) PM
12/9/2011

Existing + Construction (Phase 3) F 1292

HCM Signalized Intersection Capacity Analysis 3: Del Mar Heights Road & High Bluff Drive

HCM Signalized Intersection Capacity Analysis

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HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.
Existing + Construction (Phase 3) PM
12/9/2011

Existing + Construction (Phase 3) PM
12/9/2011

Movement	FEB	FEB	WMB	INBL	INBL
Lane Configurations	1	1	1	1	1
Volume (vph)	2145	21	14	1189	30
Volume (vph)	1800	1600	1900	1900	20
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.97	0.91	1.00
Fr.	1.00	0.85	1.00	1.00	0.85
Fl. Protected	1.00	1.00	0.95	1.00	0.95
Said. Fl. (prot)	5085	4853	3433	5085	1770
RTOR Reduction (vph)	1.00	1.00	0.95	1.00	0.95
Lane Group Flow (vph)	5086	4853	3433	5085	1770
Said. Fl. (perm)	5086	4853	3433	5085	1770
Peak-hour factor PHF	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2383	23	16	1321	33
RTOR Reduction (vph)	0	7	0	0	20
Lane Group Flow (vph)	2383	16	16	1321	33
Turn Type	Perm	Prot	Perm	Perm	Perm
Protected Phases	4	4	4	2	2
Permitted Phases	4	4	4	2	2
Adjusted Green, G(s)	38.4	38.4	0.6	43.0	39
Effective Green, G(s)	38.4	38.4	0.6	33.0	39
Adjusted g/C Ratio	0.70	0.70	0.001	0.76	0.07
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0
Lane Cap (vph)	3587	1107	38	3983	126
Wts Ratio Prot. vs Ratio Perm.	0.47	0.47	0.01	0.02	0.02
Wts Ratio Prot. vs Ratio Perm.	0.57	0.57	0.01	0.12	0.33
Uniform Delay, d _U	4.7	25	27.0	1.7	24.1
Progression Factor	1.00	1.00	1.00	1.00	1.00
Incremental Delay d _I	0.5	0.0	7.4	0.0	1.1
Delay (s)	52	25	363	18	25.8
Level of Service	A	A	C	A	C
Approach Delay (g)	5.1	5.1	5.1	2.2	2.7
Approach LOS	A	A	A	A	A
Intersections Summary					
HCM Average Control Delay				4.4	HCM Level of Service
HCM Volume-to-Capacity ratio				0.64	Sum of lost time (s)
Actuated Cycle Length (s)				54.9	ICU Level of Service
Intersection Capacity Utilization				51.4%	15
Analysis Period (min)					
Critical Lane Group					

HCM Signalized Intersection Capacity Analysis 5: Del Mar Heights Road & El Camino Real

HCM Signalized Intersection Capacity Analysis 5: Del Mar Heights Road & El Camino Real

Existing + Construction (Phase 3) PN
12/9/2011

Existing + Construction (Phase 3) PN
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Existing + Construction (Phase 3) PM
12/9/2011

HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Near Term + Construction (Phase 3) AM
2: Del Mar Heights Road & I-15 NB Ramps

HCM Signalized Intersection Capacity Analysis

Near Term + Construction (Phase 3) AM
3/5/2012

Movement	EBL	EBS	EBR	WBL	WBS	WBR	SBL	SBS	SBR
Lane Configurations									
Volume (vph)	0	1025	0	939	356	0	0	1483	930
Ideal Flow (vphpl)	1900	1900	1900	1900	5.6	5.6	0	1900	1900
Total Lost time (s)	6.3	6.3	0.97	0.97	0.91	0.91	4.0	4.0	4.0
Lane Util. Factor	0.96	0.95	1.00	0.99	0.85	1.00	0.95	0.91	0.95
Frt	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Frt Protected	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Sld. Flow (prot)	3539	3539	3430	3430	1441	1441	3539	5085	1563
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Sld. Flow (perm)	3539	3539	3430	3430	1441	1441	3539	5085	1563
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	767	1139	0	1043	373	0	1648	1033
RTOR Reduction (vph)	0	0	0	0	3	37	0	395	0
Lane Group Flow (vph)	0	767	1139	0	1077	299	0	1648	638
Turn Type	Perm							Prot	Split
Protected Phases	2	6	2	4				6	8
Permitted Phases								6	8
Actuated Green, G (s)	45.4	45.4	30.1	30.1				53.4	44.4
Effective Green, g (s)	45.4	45.4	30.1	30.1	0.34	0.34		53.4	44.4
Actuated g/C Ratio	0.52	0.52	0.52	0.52				0.44	0.44
Clearance Time (s)								0.37	0.37
Vehicle Extension (s)								0.37	0.37
Lane Grp Cap (vph)	1838	1838	1181	486				2283	704
v/s Ratio Prot	0.22	0.32	0.31	0.21				0.32	0.40
v/s Ratio Perm	0.42	0.62	0.91	0.60				0.91	0.62
v/s Ratio	12.9	14.9	27.4	23.7				27.3	30.9
Uniform Delay, d1	1.00	1.00	1.00	1.00				1.00	1.00
Progression Factor	1.00	1.00	1.00	1.00				1.00	1.00
Incremental Delay, d2	0.2	0.6	10.7	2.1				2.1	17.5
Delay (s)	13.0	16.5	38.1	28.8				38.5	32.7
Level of Service	B	B	D	C				C	E
Approach Delay (s)	13.0	15.5	36.1	28.8				36.8	33.7
Approach LOS	B	B	D	D				D	A

Intersection Summary

HCM Average Control Delay	41.9	HCM Level of Service	C
HCM Volume to Capacity ratio	0.94		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	11.9
Intersection Capacity Utilization	95.8%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Baseline

Synchro 7 - Report
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Movement	EBL	EBS	EBR	WBL	WBS	WBR	SBL	SBS	SBR
Lane Configurations									
Volume (vph)	0	1025	0	939	356	0	0	1483	930
Ideal Flow (vphpl)	1900	1900	1900	1900	5.6	5.6	4.0	4.0	4.0
Total Lost time (s)	6.3	6.3	0.97	0.97	0.91	0.91	0.95	0.95	0.95
Lane Util. Factor	0.96	0.95	1.00	0.99	0.85	1.00	0.85	1.00	0.85
Frt	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Frt Protected	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Sld. Flow (prot)	3539	3539	3430	3430	1441	1441	3539	5085	1563
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Sld. Flow (perm)	3539	3539	3430	3430	1441	1441	3539	5085	1563
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	767	1139	0	1043	373	0	1648	1033
RTOR Reduction (vph)	0	0	0	0	3	37	0	395	0
Lane Group Flow (vph)	0	767	1139	0	1077	299	0	1648	638
Turn Type	Perm							Prot	Split
Protected Phases	2	6	2	4				6	8
Permitted Phases								6	8
Actuated Green, G (s)	45.4	45.4	30.1	30.1				53.4	44.4
Effective Green, g (s)	45.4	45.4	30.1	30.1	0.34	0.34		53.4	44.4
Actuated g/C Ratio	0.52	0.52	0.52	0.52				0.44	0.44
Clearance Time (s)								0.37	0.37
Vehicle Extension (s)								0.37	0.37
Lane Grp Cap (vph)	1838	1838	1181	486				2283	704
v/s Ratio Prot	0.22	0.32	0.31	0.21				0.32	0.40
v/s Ratio Perm	0.42	0.62	0.91	0.60				0.91	0.62
v/s Ratio	12.9	14.9	27.4	23.7				27.3	30.9
Uniform Delay, d1	1.00	1.00	1.00	1.00				1.00	1.00
Progression Factor	1.00	1.00	1.00	1.00				1.00	1.00
Incremental Delay, d2	0.2	0.6	10.7	2.1				2.1	17.5
Delay (s)	13.0	16.5	38.1	28.8				38.5	32.7
Level of Service	B	B	D	C				C	E
Approach Delay (s)	13.0	15.5	36.1	28.8				36.8	33.7
Approach LOS	B	B	D	D				D	A

Movement	EBL	EBS	EBR	WBL	WBS	WBR	SBL	SBS	SBR
Lane Configurations									
Volume (vph)	0	1025	0	939	356	0	0	1483	930
Ideal Flow (vphpl)	1900	1900	1900	1900	5.6	5.6	4.0	4.0	4.0
Total Lost time (s)	6.3	6.3	0.97	0.97	0.91	0.91	0.95	0.95	0.95
Lane Util. Factor	0.96	0.95	1.00	0.99	0.85	1.00	0.85	1.00	0.85
Frt	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Frt Protected	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Sld. Flow (prot)	3539	3539	3430	3430	1441	1441	3539	5085	1563
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Sld. Flow (perm)	3539	3539	3430	3430	1441	1441	3539	5085	1563
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	767	1139	0	1043	373	0	1648	1033
RTOR Reduction (vph)	0	0	0	0	3	37	0	395	0
Lane Group Flow (vph)	0	767	1139	0	1077	299	0	1648	638
Turn Type	Perm							Prot	Split
Protected Phases	2	6	2	4				6	8
Permitted Phases								6	8
Actuated Green, G (s)	45.4	45.4	30.1	30.1				53.4	44.4
Effective Green, g (s)	45.4	45.4	30.1	30.1	0.34	0.34		53.4	44.4
Actuated g/C Ratio	0.52	0.52	0.52	0.52				0.44	0.44
Clearance Time (s)								0.37	0.37
Vehicle Extension (s)								0.37	0.37
Lane Grp Cap (vph)	1838	1838	1181	486				2283	704
v/s Ratio Prot	0.22	0.32	0.31	0.21				0.32	0.40
v/s Ratio Perm	0.42	0.62	0.91	0.60				0.91	0.62
v/s Ratio	12.9	14.9	27.4	23.7				27.3	30.9
Uniform Delay, d1	1.00	1.00	1.00	1.00				1.00	1.00
Progression Factor	1.00	1.00	1.00	1.00				1.00	1.00
Incremental Delay, d2	0.2	0.6	10.7	2.1				2.1	17.5
Delay (s)	13.0	16.5	38.1	28.8				38.5	32.7
Level of Service	B	B	D	C				C	E
Approach Delay (s)	13.0	15.5	36.1	28.8				36.8	33.7
Approach LOS	B	B	D	D				D	A

Intersection Summary

HCM Average Control Delay

HCM Volume to Capacity ratio

Actuated Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

c Critical Lane Group

HCM Level of Service

D

HCM Volume to Capacity ratio

0.94

Sum of lost time (s)

12.0

ICU Level of Service

F

15

c Critical Lane Group

Baseline

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HCM Level of Service

C

Sum of lost time (s)

11.9

ICU Level of Service

C

15

c Critical Lane Group

HCM Level of Service

C

Sum of lost time (s)

11.9

ICU Level of Service

C

15

c Critical Lane Group

HCM Level of Service

C

Sum of lost time (s)

11.9

ICU Level of Service

C

15

c Critical Lane Group

HCM Level of Service

C

Sum of lost time (s)</

HCM Signalized Intersection Capacity Analysis 3: Del Mar Heights Road & High Bluff Drive

Near Term + Construction (Phase 3) AM
3/5/2012

HCM Signalized Intersection Capacity Analysis 4: Del Mar Heights Road & First Ave.

Near Term + Construction (Phase 3) AM
3/5/2012

100

100

Baseline

Baseline

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HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Near Term + Construction (Phase 3) AM
3/5/2012

HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Near Term + Construction (Phase 3) PM
3/5/2012

Movement	EBT	EBR	WBT	WBR	NBT	NBR	SBT	SBR
Lane Configurations								
Volume (vph)	222	897	399	205	1385	95	245	104
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	0.97	0.91
Fit	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.91
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (prot)	3433	4851	3433	5036	3433	5085	1583	3433
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (perm)	3433	4851	3433	5036	3433	5085	1583	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	247	987	443	229	1539	106	272	116
RTO/R Reduction (vph)	0	105	0	0	10	0	0	71
Lane Group Flow (vph)	247	1335	0	229	1635	0	272	116
Turn Type	Prot							
Protected Phases	7	4	3	8	5	2	2	1
Permitted Phases	7	4	3	8	5	2	2	1
Actuated Green, G (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Effective Green, g (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.13	0.23
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	325	1632	413	1844	325	976	304	1062
v/s Ratio Prot	0.07	0.28	0.07	0.32	0.08	0.02	0.01	0.05
v/s Ratio Perm	0.07	0.28	0.07	0.32	0.08	0.02	0.01	0.14
v/c Ratio	0.76	0.81	0.55	0.89	0.84	0.12	0.06	0.40
Uniform Delay, d1	32.7	22.2	30.7	22.0	32.9	24.7	24.4	29.5
Progression Factor	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00
Incremental Delay, d2	10.0	3.0	1.00	1.00	1.00	1.00	1.00	1.00
Delay (s)	42.7	25.2	32.3	27.6	49.8	24.8	24.5	30.1
Level of Service	D	C	C	C	D	C	C	C
Approach Delay (s)	27.8	28.1	28.1	28.1	39.0	27.3	27.3	33.1
Approach LOS	C	C	C	D	C	C	B	C

Intersection Summary	HCM Level of Service	HCM Level of Service	HCM Average Control Delay	HCM Volume to Capacity ratio	HCM Volume to Capacity ratio	Actualized Cycle Length (s)	Actualized Cycle Length (s)	Intersection Capacity Utilization	Intersection Capacity Utilization	Analysis Period (min)	Analysis Period (min)	Baseline
	C	C	28.9	0.74	0.74	120	120	Sum of lost time (s)	Sum of lost time (s)	71.0%	71.0%	
								ICU Level of Service	ICU Level of Service	15	15	
								70.9%	70.9%			
c Critical Lane Group												

Movement	EBT	EBR	WBT	WBR	NBT	NBR	SBT	SBR
Lane Configurations								
Volume (vph)	222	897	399	205	1385	95	245	104
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	0.97	0.91
Fit	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.91
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (prot)	3433	4851	3433	5036	3433	5085	1583	3433
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (perm)	3433	4851	3433	5036	3433	5085	1583	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	247	987	443	229	1539	106	272	116
RTO/R Reduction (vph)	0	105	0	0	10	0	0	71
Lane Group Flow (vph)	247	1335	0	229	1635	0	272	116
Turn Type	Prot							
Protected Phases	7	4	3	8	5	2	2	1
Permitted Phases	7	4	3	8	5	2	2	1
Actuated Green, G (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Effective Green, g (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.13	0.23
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	325	1632	413	1844	325	976	304	1062
v/s Ratio Prot	0.07	0.28	0.07	0.32	0.08	0.02	0.01	0.14
v/s Ratio Perm	0.07	0.28	0.07	0.32	0.08	0.02	0.01	0.38
v/c Ratio	0.76	0.81	0.55	0.89	0.84	0.12	0.06	0.40
Uniform Delay, d1	32.7	22.2	30.7	22.0	32.9	24.7	24.4	29.5
Progression Factor	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00
Incremental Delay, d2	10.0	3.0	1.00	1.00	1.00	1.00	1.00	1.00
Delay (s)	42.7	25.2	32.3	27.6	49.8	24.8	24.5	30.1
Level of Service	D	C	C	C	D	C	C	C
Approach Delay (s)	27.8	28.1	28.1	28.1	39.0	27.3	27.3	33.1
Approach LOS	C	C	C	D	C	C	B	C

Movement	EBT	EBR	WBT	WBR	NBT	NBR	SBT	SBR
Lane Configurations								
Volume (vph)	222	897	399	205	1385	95	245	104
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	0.97	0.91
Fit	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.91
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (prot)	3433	4851	3433	5036	3433	5085	1583	3433
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said Flow (perm)	3433	4851	3433	5036	3433	5085	1583	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	247	987	443	229	1539	106	272	116
RTO/R Reduction (vph)	0	105	0	0	10	0	0	71
Lane Group Flow (vph)	247	1335	0	229	1635	0	272	116
Turn Type	Prot							
Protected Phases	7	4	3	8	5	2	2	1
Permitted Phases	7	4	3	8	5	2	2	1
Actuated Green, G (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Effective Green, g (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.13	0.23
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	325	1632	413	1844	325	976	304	1062
v/s Ratio Prot	0.07	0.28	0.07	0.32	0.08	0.02	0.01	0.14
v/s Ratio Perm	0.07	0.28	0.07	0.32	0.08	0.02	0.01	0.38
v/c Ratio	0.76	0.81	0.55	0.89	0.84	0.12	0.06	0.40
Uniform Delay, d1	32.7	22.2	30.7	22.0	32.9	24.7	24.4	29.5
Progression Factor	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00	-1.00
Incremental Delay, d2	10.0	3.0	1.00	1.00	1.00	1.00	1.00	1.00
Delay (s)	42.7	25.2	32.3	27.6	49.8	24.8	24.5	30.1
Level of Service	D	C	C	C	D	C	C	C
Approach Delay (s)	27.8	28.1	28.1	28.1	39.0	27.3	27.3	33.1
Approach LOS	C	C	C	D	C	C	B	C

Intersection Summary	HCM Level of Service	HCM Level of Service	HCM Average Control Delay	HCM Volume to Capacity ratio	HCM Volume to Capacity ratio	Actualized Cycle Length (s)	Actualized Cycle Length (s)	Intersection Capacity Utilization	Intersection Capacity Utilization	Analysis Period (min)	Analysis Period (min)	Baseline
	C	C	28.9	0.74	0.74	120	120	Sum of lost time (s)	Sum of lost time (s)	71.0%	71.0%	
								ICU Level of Service	ICU Level of Service	70.9%	70.9%	
								70.9%	70.9%	15	15	
c Critical Lane Group												

Intersection Summary	HCM Level of Service	HCM Level of Service	HCM Average Control Delay	HCM Volume to Capacity ratio	HCM Volume to Capacity ratio	Actualized Cycle Length (s)	Actualized Cycle Length (s)	Intersection Capacity Utilization	Intersection Capacity Utilization	Analysis Period (min)	Analysis Period (min)	Baseline
	C	C	28.9	0.74	0.74	120	120	Sum of lost time (s)	Sum of lost time (s)	71.0%	71.0%	
								ICU Level of Service	ICU Level of Service	70.9%	70.9%	
								70.9%	7			

HCM Signalized Intersection Capacity Analysis
Near Term + Construction (Phase 3) PM
3/5/2012

HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Near Term + Construction (Phase 3) PM
3/5/2012

2: Del Mar Heights Road & I-15 NB Ramps

Movement	T-BBL	T-BBR	W-BBL	W-BBR	N-BBL	N-BBR	S-BBL	S-BBR
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	242	1526	0	1205	902	649	24	811
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	1.00	0.95	0.91	0.95	0.95
Frt	1.00	1.00	1.00	0.85	1.00	0.90	1.00	0.90
Frt Protected	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Satd. Flow (prot)	3433	3539	5085	1583	1681	1503	1504	1504
Frt Permitted	0.95	1.00	1.00	0.95	0.98	1.00	1.00	1.00
Satd. Flow (perm)	3433	3539	5085	1583	1681	1503	1504	1504
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1696	0	1339	1002	721	27	901
R/TOR Reduction (vph)	0	0	0	0	509	0	6	6
Lane Group Flow (vph)	269	1696	0	1339	493	570	541	526
Turn Type	Prot							
Protected Phases	5	2	6	6	8	8	8	8
Permitted Phases								
Actuated Green, G (s)	11.3	62.2	46.9	46.9	49.8	49.8	49.8	49.8
Effective Green, g (s)	11.3	62.2	46.9	46.9	49.8	49.8	49.8	49.8
Actuated g/C Ratio	0.09	0.52	0.39	0.39	0.42	0.42	0.42	0.42
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap (vph)	323	1834	1987	619	698	624	624	624
v/s Ratio Prot	0.08	0.48	0.26	0.31	0.34	0.36	0.35	0.35
v/s Ratio Perm								
w/s Ratio	0.83	0.92	0.67	0.80	0.82	0.87	0.84	0.84
Uniform Delay, d ₁	53.4	26.7	30.2	32.3	31.1	32.1	31.6	31.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d ₂	16.6	9.4	1.8	10.2	7.3	12.2	10.1	10.1
Delay (s)	70.0	36.2	32.1	42.5	38.4	44.2	41.7	41.7
Level of Service	E	D	C	D	D	D	B	A
Approach Delay (s)	40.8	36.6	36.6	41.4	41.4	41.4	41.4	41.4
Approach LOS	D	D	D	D	D	D	D	D

Intersection Summary								
HCM Average Control Delay	39.3	HCM Level of Service	D					
HCM Volume to Capacity ratio	0.90							
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	8.0					
Intersection Capacity Utilization (%)	98.6%	ICU Level of Service	F					
Analysis Period (min)	15							
c Critical Lane Group								

Movement	T-BBL	T-BBR	W-BBL	W-BBR	N-BBL	N-BBR	S-BBL	S-BBR
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	249	2110	259	15	1383	29	637	28
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	0.95	1.00	1.00	0.91	1.00
Frt	1.00	1.00	1.00	0.85	1.00	1.00	1.00	1.00
Frt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	3433	3539	5085	1583	1681	1503	1504	1504
Frt Permitted	0.95	1.00	1.00	0.95	0.98	1.00	1.00	1.00
Satd. Flow (perm)	3433	3539	5085	1583	1681	1503	1504	1504
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	277	2344	288	17	1537	32	708	74
R/TOR Reduction (vph)	0	0	0	0	93	0	2	0
Lane Group Flow (vph)	277	2344	195	17	1567	0	708	112
Turn Type	Prot							
Protected Phases	7	4	3	8	7	4	3	8
Permitted Phases								
Actuated Green, G (s)	17.7	54.6	54.6	1.7	38.6	22.1	25.1	3.9
Effective Green, g (s)	17.7	54.6	54.6	1.7	38.6	22.1	25.1	3.9
Actuated g/C Ratio	0.17	0.54	0.54	0.02	0.38	0.22	0.25	0.04
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap (vph)	309	2741	853	30	1932	749	788	68
v/s Ratio Prot	0.16	0.46	0.12	0.01	0.31	0.21	0.04	0.02
v/s Ratio Perm								
w/s Ratio	0.90	0.86	0.23	0.57	0.81	0.95	0.14	0.46
Uniform Delay, d ₁	40.9	20.0	12.3	49.4	28.1	39.0	29.7	47.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d ₂	26.5	2.8	0.1	22.3	2.7	20.5	0.1	4.8
Delay (s)	67.4	22.8	12.4	71.7	30.8	39.5	29.8	52.4
Level of Service	E	C	B	E	C	E	C	D
Approach Delay (s)	26.0	31.2	31.2	31.2	31.2	32.3	32.3	46.3
Approach LOS	C	C	C	C	C	D	D	D

Intersection Summary								
HCM Average Control Delay	32.5	HCM Level of Service	C					
HCM Volume to Capacity ratio	0.83							
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	101.3					
Intersection Capacity Utilization (%)	78.9%	ICU Level of Service	D					
Analysis Period (min)	15							
c Critical Lane Group								

Baseline

Synchro 7 - Report
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HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.

Near Term + Construction (Phase 3) PM
5: Del Mar Heights Road & El Camino Real

HCM Signalized Intersection Capacity Analysis
Near Term + Construction (Phase 3) PM
3/25/2012

Movement	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	2301	21	14	1588	30	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.91	1.00	1.00	0.91
Fit	1.00	0.85	1.00	1.00	0.85	1.00
Fit Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	5085	1683	3433	5085	1770	1583
Fit Permitted	1.00	0.95	1.00	0.95	1.00	0.95
Adj. Flow (perm)	5085	1583	3433	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vhpl)	2567	73	16	1764	33	22
RTO/R Reduction (vhpl)	0	7	0	0	0	0
Lane Group Flow (vph)	2557	16	16	1764	33	2
Turn Type	Pem	Pem	Pem	Pem	Pem	Pem
Protected Phases	4	3	8	2	2	2
Permitted Phases	4	4	2	2	2	2
Actuated Green, G (s)	39.4	39.4	0.7	44.1	3.9	3.9
Effective Green, g (s)	39.4	39.4	0.7	44.1	3.9	3.9
Actuated g/C Ratio	0.70	0.70	0.01	0.78	0.07	0.07
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3578	1114	43	4004	123	110
Vs Ratio Prot	0.50	0.00	0.35	0.02	0.00	0.00
Vs Ratio Perm	0.01	0.00	0.00	0.00	0.00	0.00
Vc Ratio	0.71	0.01	0.37	0.44	0.27	0.01
Uniform Delay, d1	4.9	2.5	27.4	1.9	24.7	24.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	0.7	0.0	5.4	0.1	1.2
Delay (s)	5.6	2.5	32.8	2.0	25.9	24.3
Level of Service	A	A	C	A	C	C
Approach Delay (s)	5.6	2.5	32.8	2.0	25.9	24.3
Approach LOS	A	A	C	A	C	C

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	4.5	HCM Level of Service	A
HCM Volume to Capacity ratio	0.68	HCM Volume to Capacity ratio	D
Actuated Cycle Length (s)	56.0	Sum of lost time (s)	120
Intersection Capacity Utilization	54.2%	ICU Level of Service	A
Analysis Period (min)	15	Analysis Period (min)	15
c Critical Lane Group			

Movement	EBL	EBR	WBL	WBR	NBL	NBR	SBT	SEB
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	2301	21	14	1588	30	20	181	453
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	1.00	1.00	0.91	0.91	0.91	0.91
Fit	1.00	0.85	1.00	1.00	0.85	1.00	0.85	0.92
Fit Protected	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	5085	1683	3433	5085	1770	1583	3433	3433
Fit Permitted	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Adj. Flow (vhpl)	2567	73	16	1764	33	22	3433	3433
RTO/R Reduction (vhpl)	0	7	0	0	0	0	0	0
Lane Group Flow (vph)	2557	16	16	1764	33	2	3433	3433
Turn Type	Pem							
Protected Phases	4	3	8	2	2	2	2	2
Permitted Phases	4	4	2	2	2	2	2	2
Actuated Green, G (s)	39.4	39.4	0.7	44.1	3.9	3.9	10.1	15.4
Effective Green, g (s)	39.4	39.4	0.7	44.1	3.9	3.9	10.1	15.4
Actuated g/C Ratio	0.70	0.70	0.01	0.78	0.07	0.07	0.12	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3578	1114	43	4004	123	110	1694	417
Vs Ratio Prot	0.50	0.00	0.35	0.02	0.00	0.00	0.20	0.04
Vs Ratio Perm	0.01	0.00	0.00	0.00	0.00	0.00	0.09	0.04
Vc Ratio	0.71	0.01	0.37	0.44	0.27	0.01	1.21	0.50
Uniform Delay, d1	4.9	2.5	27.4	1.9	24.7	24.3	36.5	30.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	0.7	0.0	5.4	0.1	1.2	6.7	0.6
Delay (s)	5.6	2.5	32.8	2.0	25.9	24.3	46.1	23.1
Level of Service	A	A	C	A	C	C	F	D
Approach Delay (s)	5.6	2.5	32.8	2.0	25.9	24.3	46.1	23.1
Approach LOS	A	A	C	A	C	C	E	C

Intersection Summary		HCM Level of Service	
HCM Average Control Delay	4.5	HCM Level of Service	A
HCM Volume to Capacity ratio	0.68	HCM Volume to Capacity ratio	D
Actuated Cycle Length (s)	56.0	Sum of lost time (s)	120
Intersection Capacity Utilization	54.2%	ICU Level of Service	A
Analysis Period (min)	15	Analysis Period (min)	15
c Critical Lane Group			

Baseline

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HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps
Existing + Construction (Phase 1&2) AM
12/29/2011

Existing + Construction (Phase 1&2) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis 1: Del Mar Heights Bd & I-15 SB Ramon

HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps
Existing + Construction (Phase 1&2) AM
12/9/2011

Existing + Construction (Phase 1 & 2) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis 2: Del Mar Heights Road & I-15 NB Ramps

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HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & Third Ave.

Movement	EBE	SEBR	WBET	WBR	NBET	NBR	SBET	SBR
Lane Configurations								
Volume (vph)	108	1222	674	92	1229	59	195	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.97	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.96	1.00	0.95	1.00	1.00
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	20	1396	749	102	2332	66	217	11
RTOR Reduction (vph)	0	0	378	0	3433	322	100	0
Lane Group Flow (vph)	120	1356	371	102	2694	0	217	13
Turn Type	Prot							
Protected Phases	7	24	4	3	8	5	2	1
Permitted Phases	6	24	5	3	6	4	3	2
Actuated Green, g (s)	7.9	41.3	41.3	7.9	11.1	7.0	11.1	7.0
Effective Green, g (s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	12.9
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.46	0.08	0.46	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap (vph)	159	2306	73	135	2306	273	409	259
vs.Ratio,Prot	6.07	0.27	0.23	0.06	0.04	0.06	0.03	0.13
vs.Ratio,Permit	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
yc.Ratio	0.76	0.57	0.50	0.76	0.91	0.78	0.93	0.34
Uniform Delay, d1	39.1	16.9	16.2	39.8	22.2	30.8	33.1	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	5.7	14.7	0.0	0.2
Delay (s)	57.3	1.2	1.67	60.9	27.9	34.5	33.3	34.6
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	102	102	29.5	523	362	362	362	362
Approach LOS	B	C	D	D	D	D	D	D

Intersection Summary	C	C	C	C	C	C	C	C
HCM Average Control Delay	26.8	HCM Level of Service	C	C	C	C	C	C
HCM Volume to Capacity ratio	0.82	Actual Cycle Length (s)	160	Sum of lost time (s)	71.0%	ICU Level of Service	C	C
Actualized Cycle Length (s)	86.0	Intersection Capacity Utilization	71.0%	Analysis Period (min)	15	Critical Lane Group	C	C
Intersection Capacity Utilization	71.0%	Analysis Period (min)	15	Critical Lane Group	C	C	C	C

Existing + Construction (Phase 1&2) AM
12/29/2011

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & Third Ave.

Movement	ESTATE	TERRELL	WILLIAMS	NEBB	SBEST	SBTER	SBWILL	SBNEB
Lane Configurations								
Volume (vph)	108	1222	674	92	1229	59	195	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.91	1.00	1.00	0.97	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.96	1.00	0.95	1.00	1.00
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	20	1396	749	102	2332	66	217	11
RTOR Reduction (vph)	0	0	378	0	3433	322	100	0
Lane Group Flow (vph)	120	1356	371	102	2694	0	217	13
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	24	4	3	8	5	2	1
Permitted Phases	6	24	5	3	6	4	3	2
Actuated Green, g (s)	7.9	41.3	41.3	7.9	11.1	7.0	11.1	7.0
Effective Green, g (s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	12.9
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.46	0.08	0.46	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap (vph)	159	2306	73	135	2306	273	409	259
vs.Ratio,Prot	6.07	0.27	0.23	0.06	0.04	0.06	0.03	0.13
vs.Ratio,Permit	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
yc.Ratio	0.76	0.57	0.50	0.76	0.91	0.78	0.93	0.34
Uniform Delay, d1	39.1	16.9	16.2	39.8	22.2	30.8	33.1	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	5.7	14.7	0.0	0.2
Delay (s)	57.3	1.2	1.67	60.9	27.9	34.5	33.3	34.6
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	102	102	29.5	523	362	362	362	362
Approach LOS	B	C	D	D	D	D	D	D

Movement	ESTATE	TERRELL	WILLIAMS	NEBB	SBEST	SBTER	SBWILL	SBNEB
Lane Configurations								
Volume (vph)	108	1222	674	92	1229	59	195	10
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.91	1.00	1.00	0.97	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.96	1.00	0.95	1.00	1.00
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	20	1396	749	102	2332	66	217	11
RTOR Reduction (vph)	0	0	378	0	3433	322	100	0
Lane Group Flow (vph)	120	1356	371	102	2694	0	217	13
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	24	4	3	8	5	2	1
Permitted Phases	6	24	5	3	6	4	3	2
Actuated Green, g (s)	7.9	41.3	41.3	7.9	11.1	7.0	11.1	7.0
Effective Green, g (s)	7.9	41.3	41.3	6.7	40.1	7.0	11.1	12.9
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.46	0.08	0.46	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Cap (vph)	159	2306	73	135	2306	273	409	259
vs.Ratio,Prot	6.07	0.27	0.23	0.06	0.04	0.06	0.03	0.13
vs.Ratio,Permit	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
yc.Ratio	0.76	0.57	0.50	0.76	0.91	0.78	0.93	0.34
Uniform Delay, d1	39.1	16.9	16.2	39.8	22.2	30.8	33.1	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	18.2	0.3	0.5	21.1	5.7	14.7	0.0	0.2
Delay (s)	57.3	1.2	1.67	60.9	27.9	34.5	33.3	34.6
Level of Service	E	B	B	C	D	C	C	D
Approach Delay (s)	102	102	29.5	523	362	362	362	362
Approach LOS	B	C	D	D	D	D	D	D

Baseline

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HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Existing + Construction (Phase 1&2) AM
12/9/2011
1: Del Mar Heights Rd. & I-15 SB Ramps

Movement		EB		WB		NB		WB		SB		EB		WB		NB		WB	
Turn Type	Prot.																		
Protected Phases																			
Permitted Phases																			
Actuated Green, G(s)	8.7	24.1	9.7	25.1	7.0	13.2	9.5	15.7	7.0	13.2	9.5	15.7	7.0	13.2	9.5	15.7	7.0	13.2	9.5
Effective Green, g(s)	8.7	24.1	9.7	25.1	7.0	13.2	9.5	15.7											
Actuated g/C Ratio	0.12	0.38	0.13	0.36	0.10	0.18	0.13	0.22											
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	412	1841	459	1744	391	926	298	1004											
Vis Ratio Prot.	0.07	0.23	0.06	0.22	0.07	0.01	0.05	0.12											
Vis Ratio Perm																			
Vis Ratio																			
Uniform Delay, d1	30.2	21.1	29.0	22.6	31.9	24.8	24.5	28.9	25.3										
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00										
Incremental Delay, d2	2.1	1.4	0.7	0.76	9.3	0.1	0.1	0.6	0.7										
Delay (s)	32.3	22.8	28.5	30.3	41.2	24.8	24.6	29.4	25.9										
Level of Service	C	C	C	C	D	C	C	C	C										
Approach LOS	C	C	C	C	C	C	C	C	C										
Intersection Summary																			
HCM Average Control Delay																			
HCM Volume to Capacity ratio																			
Adjusted Cycle Length (s)																			
Intersection Capacity Utilization																			
Analysis Period (min)																			
Critical Lane Group																			

Baseline
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Movement		EB		WB		NB		WB		SB		EB		WB		NB		WB	
Turn Type	Prot.																		
Protected Phases																			
Permitted Phases																			
Actuated Green, G(s)	8.7	24.1	9.7	25.1	7.0	13.2	9.5	15.7	7.0	13.2	9.5	15.7	7.0	13.2	9.5	15.7	7.0	13.2	9.5
Effective Green, g(s)	8.7	24.1	9.7	25.1	7.0	13.2	9.5	15.7											
Actuated g/C Ratio	0.12	0.38	0.13	0.36	0.10	0.18	0.13	0.22											
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	412	1841	459	1744	391	926	298	1004											
Vis Ratio Prot.	0.07	0.23	0.06	0.22	0.07	0.01	0.05	0.12											
Vis Ratio Perm																			
Vis Ratio																			
Uniform Delay, d1	30.2	21.1	29.0	22.6	31.9	24.8	24.5	28.9	25.3										
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00										
Incremental Delay, d2	2.1	1.4	0.7	0.76	9.3	0.1	0.1	0.6	0.7										
Delay (s)	32.3	22.8	28.5	30.3	41.2	24.8	24.6	29.4	25.9										
Level of Service	C	C	C	C	D	C	C	C	C										
Approach LOS	C	C	C	C	C	C	C	C	C										
Intersection Summary																			
HCM Average Control Delay																			
HCM Volume to Capacity ratio																			
Adjusted Cycle Length (s)																			
Intersection Capacity Utilization																			
Analysis Period (min)																			
Critical Lane Group																			

Movement		EB		WB		NB		WB		SB		EB		WB		NB		WB	
Turn Type	Prot.																		
Protected Phases																			
Permitted Phases																			
Actuated Green, G(s)	8.7	24.1	9.7	25.1	7.0	13.2	9.5	15.7	7.0	13.2	9.5	15.7	7.0	13.2	9.5	15.7	7.0	13.2	9.5
Effective Green, g(s)	8.7	24.1	9.7	25.1	7.0	13.2	9.5	15.7											
Actuated g/C Ratio	0.12	0.38	0.13	0.36	0.10	0.18	0.13	0.22											
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	412	1841	459	1744	391	926	298	1004											
Vis Ratio Prot.	0.07	0.23	0.06	0.22	0.07	0.01	0.05	0.12											
Vis Ratio Perm																			
Vis Ratio																			
Uniform Delay, d1	30.2	21.1	29.0	22.6	31.9	24.8	24.5	28.9	25.3										
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00										
Incremental Delay, d2	2.1	1.4	0.7	0.76	9.3	0.1	0.1	0.6	0.7										
Delay (s)	32.3	22.8	28.5	30.3	41.2	24.8	24.6	29.4	25.9										
Level of Service	C	C	C	C	D	C	C	C	C										
Approach LOS	C	C	C	C	C	C	C	C	C										
Intersection Summary																			
HCM Average Control Delay																			
HCM Volume to Capacity ratio																			
Adjusted Cycle Length (s)																			
Intersection Capacity Utilization																			
Analysis Period (min)																			
Critical Lane Group																			

Movement		EB		WB		NB		WB		SB		EB		WB		NB		WB		SB	
Turn Type	Prot.																				

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HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

HCM Signalized Intersection Capacity Analysis Near Term + Construction (Phase 1&2) AM
4: De Mar Heights Road & Third Ave.
3/5/2012

Movement	EBL	EBT	WBL	WBT	NBL	NBT	SBT	SBT
Lane Configurations								
Volume (vph)	111	1381	694	95	1897	61	201	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.97	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	0.85
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1683	1770	5061	3433	3292	1770
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1683	1770	5061	3433	3292	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1534	771	106	2108	68	223	11
RTOR Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1534	396	106	2172	0	223	13
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.15
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap. (vph)	160	2376	740	134	2290	272	411	264
Ws Ratio Prot	c0.07	0.30	0.25	0.06	c0.43	c0.06	0.05	0.04
Ws Ratio Perm								
Wf Ratio	0.77	0.65	0.54	0.79	0.85	0.82	0.03	0.34
Uniform Delay, d1	39.3	18.0	16.7	40.2	23.2	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.6	0.7	26.4	9.3	17.3	0.0	0.8
Delay (s)	58.9	18.6	17.5	66.6	32.5	57.4	38.9	34.5
Level of Service	E	B	B	C	C	E	C	C
Approach Delay (s)	20.3	20.3	34.1	34.1	34.1	56.0	37.0	37.0
Approach LOS	C	C	D	C	D	D	D	D

Intersection Summary	HCM Average Control Delay	4.4	HCM Level of Service	C
HCM Volume to Capacity ratio	0.55			
Actuated Cycle Length (s)	49.3			
Intersection Capacity Utilization	A			
Analysis Period (min)	15			
c Critical Lane Group				

Movement	EFL	ETB	WFL	WBT	NFL	NBT	SBT	SBT
Lane Configurations								
Volume (vph)	111	1381	694	95	1897	61	201	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	0.91	0.97	0.95	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	0.85
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1683	1770	5061	3433	3292	1770
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1683	1770	5061	3433	3292	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1534	771	106	2108	68	223	11
RTOR Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1534	396	106	2172	0	223	13
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.15
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap. (vph)	160	2376	740	134	2290	272	411	264
Ws Ratio Prot	c0.07	0.30	0.25	0.06	c0.43	c0.06	0.05	0.04
Ws Ratio Perm								
Wf Ratio	0.77	0.65	0.54	0.79	0.85	0.82	0.03	0.34
Uniform Delay, d1	39.3	18.0	16.7	40.2	23.2	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.6	0.7	26.4	9.3	17.3	0.0	0.8
Delay (s)	58.9	18.6	17.5	66.6	32.5	57.4	38.9	34.5
Level of Service	E	B	B	C	C	E	C	C
Approach Delay (s)	20.3	20.3	34.1	34.1	34.1	56.0	37.0	37.0
Approach LOS	C	C	D	C	D	D	D	D

Movement	EER	EBR	WBR	NBR	NBR	NBR	NBR	NBR
Lane Configurations								
Volume (vph)	111	1381	694	95	1897	61	201	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit	1.00	1.00	0.85	1.00	1.00	0.92	1.00	0.85
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1683	1770	5061	3433	3292	1770
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1683	1770	5061	3433	3292	1770
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1534	771	106	2108	68	223	11
RTOR Reduction (vph)	0	0	375	0	4	0	12	0
Lane Group Flow (vph)	123	1534	396	106	2172	0	223	13
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2
Actuated g/C Ratio	0.09	0.47	0.47	0.08	0.45	0.08	0.13	0.15
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap. (vph)	160	2376	740	134	2290	272	411	264
Ws Ratio Prot	c0.07	0.30	0.25	0.06	c0.43	c0.06	0.05	0.04
Ws Ratio Perm								
Wf Ratio	0.77	0.65	0.54	0.79	0.85	0.82	0.03	0.34
Uniform Delay, d1	39.3	18.0	16.7	40.2	23.2	40.1	33.8	33.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.6	0.7	26.4	9.3	17.3	0.0	0.8
Delay (s)	58.9	18.6	17.5	66.6	32.5	57.4	38.9	34.5
Level of Service	E	B	B	C	C	C	C	D
Approach Delay (s)	20.3	20.3	34.1	34.1	34.1	56.0	37.0	37.0
Approach LOS	C	C	D	C	D	D	D	D

Baseline

Synchro 7 - Report
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Synchro 7 - Report
Page 1

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Near Term + Construction (Phase 1&2) AM
3/5/2012
HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	223	300	341	202	1367	95	244	103	79	164	306	421
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91	0.97
Fit	1.00	0.96	1.00	0.99	1.00	1.00	0.85	1.00	0.91	1.00	0.85	1.00
FIT Protected	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (proj)	3433	4876	3433	5036	3433	5085	1583	3433	4843	3539	3539	3430
FIT Permitted	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (perm)	3433	4876	3433	5036	3433	5085	1583	3433	4843	3539	3539	3430
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	248	1000	379	224	1541	106	271	114	88	182	340	468
R/TOR Reduction (vph)	0	89	0	0	10	0	0	71	0	160	0	0
Lane Group Flow (vph)	248	1290	0	224	1637	0	271	114	17	182	648	0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6	2	6	6	4
Permitted Phases	7	252	8.8	27.0	7.0	14.2	14.2	9.7	16.9	45.7	45.7	4
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.2	14.2	9.7	16.9	45.7	45.7	29.9
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	24.7	24.7	29.4	25.5	45.7	45.7	29.9
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.19	0.13	0.23	0.52	0.52	0.34
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	325	1663	409	1840	325	977	304	451	1082	1868	1848	1172
v/s Ratio Proj	c0.07	0.26	0.07	0.33	c0.08	0.02	0.01	0.05	c0.14	0.29	0.37	0.30
v/s Ratio Perm	0.76	0.78	0.55	0.89	0.83	0.12	0.06	0.40	0.02dr	0.55	0.72	0.88
v/s Ratio	32.6	21.8	30.7	22.0	32.9	24.7	24.4	29.4	25.5	14.0	16.0	27.2
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	23.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	10.2	23	15	5.7	16.6	0.1	0.1	0.6	1.0	0.3	1.4	8.2
Delay (s)	42.8	24.2	32.2	27.8	49.1	24.7	24.6	30.0	26.6	14.3	17.3	35.4
Level of Service	D	C	C	C	D	C	C	C	C	B	B	D
Approach Delay (s)	27.0	27.0	28.3	28.3	38.8	27.2	27.2	33.0	33.0	14.3	17.3	33.0
Approach LOS	C	C	C	C	D	C	C	C	C	B	B	C

Intersection Summary

HCM Average Control Delay	28.7	HCM Level of Service	C
HCM Volume to Capacity ratio	0.74	Sum of lost time (s)	12.0
Actuated Cycle Length (s)	73.9	ICU Level of Service	C
Intersection Capacity Utilization	71.0%	Analysis Period (min)	15
Analysis Period (min)	dr	Defacto Right Lane. Recode with 1 though lane as a right lane.	c Critical Lane Group

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	223	300	341	202	1367	95	244	103	79	164	306	421
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91	0.97
Fit	1.00	0.96	1.00	0.99	1.00	1.00	0.85	1.00	0.91	1.00	0.85	1.00
FIT Protected	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (proj)	3433	4876	3433	5036	3433	5085	1583	3433	4843	3539	3539	3430
FIT Permitted	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (perm)	3433	4876	3433	5036	3433	5085	1583	3433	4843	3539	3539	3430
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	248	1000	379	224	1541	106	271	114	88	182	340	468
R/TOR Reduction (vph)	0	89	0	0	10	0	0	71	0	160	0	0
Lane Group Flow (vph)	248	1290	0	224	1637	0	271	114	17	182	648	0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6	2	6	6	4
Permitted Phases	7	252	8.8	27.0	7.0	14.2	14.2	9.7	16.9	45.7	45.7	4
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	14.2	14.2	9.7	16.9	45.7	45.7	29.9
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	24.7	24.7	29.4	25.5	45.7	45.7	29.9
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.19	0.13	0.23	0.52	0.52	0.34
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	5.6
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	325	1663	409	1840	325	977	304	451	1082	1868	1848	1172
v/s Ratio Proj	c0.07	0.26	0.07	0.33	c0.08	0.02	0.01	0.05	c0.14	0.29	0.37	0.30
v/s Ratio Perm	0.76	0.78	0.55	0.89	0.83	0.12	0.06	0.40	0.02dr	0.55	0.72	0.88
v/s Ratio	32.6	21.8	30.7	22.0	32.9	24.7	24.4	29.4	25.5	14.0	16.0	27.2
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	23.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	10.2	23	15	5.7	16.6	0.1	0.1	0.6	1.0	0.3	1.4	8.2
Delay (s)	42.8	24.2	32.2	27.8	49.1	24.7	24.6	30.0	26.6	14.3	17.3	35.4
Level of Service	D	C	C	C	D	C	C	C	C	B	B	D
Approach Delay (s)	27.0	27.0	28.3	28.3	38.8	27.2	27.2	32.2	32.2	14.3	17.3	33.0
Approach LOS	C	C	C	C	D	C	C	C	C	B	B	C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	223	300	341	202	1367	95	244	103	79	164	306	421
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.97	0.91	0.97
Fit	1.00	0.96	1.00	0.99	1.00	1.00	0.85	1.00	0.91	1.00	0.85	1.00
FIT Protected	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (proj)	3433	4876	3433	5036	3433	5085	1583	3433	4843	3539	3539	3430
FIT Permitted	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (perm)	3433	4876	3433	5036	3433	5085	1583	3433	4843	3539	3539	3430
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	248	1000	379	224	1541	106	271	114	88	182	340	468
R/TOR Reduction (vph)	0	89	0	0	10	0	0	71	0	160	0	0
Lane Group Flow (vph)	248	1290	0	224	1637	0	271	114	17	182	648	0
Turn Type	Prot											
Protected Phases	7	4	3	8	5	2	1	6	2	6	6	4
Permitted Phases	7	252	8.8	27.0	7.0	14.2	14.2	9.7	16.9	45.7	45.7	4
Actuated Green, G (s)	7.0	25.2	8.8	27.0	7.0	24.7	24.7	29.4	25.5	45.7	45.7	29.9
Effective Green, g (s)	7.0	25.2	8.8	27.0	7.0	24.7	24.7	29.4	25.5	45.7	45.7	29.9
Actuated g/C Ratio	0.09	0										

HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & Third Ave.
3/5/2012

HCM Signalized Intersection Capacity Analysis
Near Term + Construction (Phase 1&2) PM
5: Del Mar Heights Road & El Camino Real
3/5/2012

Movement	EBI	EBR	WB	WBT	NBL	NBR
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	2985	32	21	1527	44	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.91	0.97	1.00	0.95
Fit	1.00	1.00	1.00	0.95	1.00	0.95
Fit Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	5085	1583	1770	5085	3433	1583
Fit Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	5085	1583	1770	5085	3433	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2539	36	23	1697	49	32
RTOR Reduction (vph)	0	11	0	0	30	0
Lane Group Flow (vph)	2539	25	23	1697	49	2
Turn Type	Perm	Prot	Prot	Perm	Prot	Prot
Protected Phases	4	3	8	2	3	8
Permitted Phases	4	4	2	2	4	5
Actuated Green, G (s)	38.8	38.8	0.7	43.5	3.7	3.7
Effective Green, g (s)	38.8	38.8	0.7	43.5	3.7	3.7
Actuated g/C Ratio	0.70	0.70	0.01	0.79	0.07	0.07
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3574	1113	22	4007	230	106
v/s Ratio Prot.	0.50	0.01	0.01	0.33	0.01	0.01
v/s Ratio Perm	0.02	0.00	0.00	0.00	0.00	0.00
v/c Ratio	0.71	0.02	0.05	0.42	0.21	0.02
Uniform Delay, d1	4.9	2.5	27.3	1.9	24.4	24.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	0.0	206.7	0.1	0.5	0.1
Delay (s)	5.5	2.5	233.9	1.9	24.8	24.1
Level of Service	A	A	F	A	C	C
Approach Delay (s)	5.5	5.0	5.0	24.6	5.0	24.6
Approach LOS	A	A	A	C	C	C
Intersection Summary						
HCM Average Control Delay	5.7	HCM Level of Service		C	HCM Level of Service	
HCM Volume to Capacity ratio	0.68	HCM Volume to Capacity ratio		C	HCM Volume to Capacity ratio	
Actuated Cycle Length (s)	55.2	Actuated Cycle Length (s)		12.0	Actuated Cycle Length (s)	
Intersection Capacity Utilization (%)	54.1%	Intersection Capacity Utilization (%)		A	Intersection Capacity Utilization (%)	
Analysis Period (min)	15	Analysis Period (min)		15	Analysis Period (min)	
c - Critical Lane Group		c - Critical Lane Group			c - Critical Lane Group	

Movement	EBI	EBR	WB	WBT	NBL	NBR	SBT	SBR
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	456	1398	402	107	768	181	396	416
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	1.00	0.97	0.91	0.91
Fit	1.00	0.97	1.00	0.97	1.00	0.97	1.00	0.97
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3433	4915	3433	4940	3433	5085	1583	3433
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3433	4915	3433	4940	3433	5085	1583	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	507	1564	447	119	853	201	440	462
RTOR Reduction (vph)	0	53	0	41	0	0	121	0
Lane Group Flow (vph)	507	1948	0	119	1013	0	440	462
Turn Type	Pilot	Pilot	Pilot	Pilot	Pilot	Pilot	Pilot	Pilot
Protected Phases	7	4	3	8	3	8	5	2
Permitted Phases	7	4	3	8	3	8	5	2
Actuated Green, G (s)	16.5	40.4	4.6	28.5	10.1	15.2	15.2	6.7
Effective Green, g (s)	16.5	40.4	4.6	28.5	10.1	15.2	15.2	6.7
Actuated g/C Ratio	0.20	0.48	0.06	0.34	0.12	0.18	0.18	0.14
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	683	2395	190	1698	418	932	290	277
v/s Ratio Prot.	0.0119	0.040	0.03	0.21	0.13	0.09	0.10	0.04
v/s Ratio Perm								
v/c Ratio	0.74	0.81	0.63	0.60	1.05	0.50	0.55	0.61
Uniform Delay, d1	31.2	18.0	38.3	22.5	36.4	30.4	30.8	31.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	4.4	2.2	6.3	6.6	58.5	0.4	2.3	3.7
Delay (s)	35.6	20.3	44.6	23.0	94.9	30.8	33.0	32.1
Level of Service	D	C	C	D	F	C	D	C
Approach Delay (s)	23.4	25.2	25.2	25.2	55.2	34.7	34.7	34.7
Approach LOS	C	C	C	C	E	C	C	C
Intersection Summary								
HCM Average Control Delay	31.9	HCM Level of Service		C	HCM Level of Service		C	C
HCM Volume to Capacity ratio	0.77	HCM Volume to Capacity ratio		C	HCM Volume to Capacity ratio		C	C
Actuated Cycle Length (s)	82.9	Actuated Cycle Length (s)		12.0	Actuated Cycle Length (s)		C	C
Intersection Capacity Utilization (%)	71.4%	Intersection Capacity Utilization (%)		15	Intersection Capacity Utilization (%)		C	C
Analysis Period (min)		Analysis Period (min)			Analysis Period (min)			
c - Critical Lane Group		c - Critical Lane Group			c - Critical Lane Group			

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HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive
Existing + Construction (Phase 1-3) AM
12/9/2011

Existing + Construction (Phase 1-3) AM
12/9/2011

HCM Signalized Intersection Capacity Analysis
4. D-1-Maughan's Board & First Ave

HCM Signalized Intersection Capacity

Movement	EBR	WB	WT
	EBR	WB	WT
Lane Configurations	144	144	144
Volume (vph)	1271	43	20
ideal Flow (vph)	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.97
Fit	1.00	0.85	1.00
Fit Protected	1.00	1.00	0.95
Safe Flow (vph)	5085	1883	3433
Fit Permitted	1.00	1.00	0.95
Safe Flow (perm)	5085	1883	3433
Peak-hour Factor PHF	0.90	0.90	0.90
Adj. Flow (vph)	1412	40	31
RTR (Ratio) (vph)	0	13	0
Lane Group Flow (vph)	1412	35	31
Turn Type	Perm	Prol	3
Protected Phases	4	3	6
Permitted Phases	4	3	6
Activated Green, G (s)	47.6	47.6	1.8
Effective Green, g (s)	47.6	47.6	1.0
Activated G/C Ratio	0.73	0.73	0.03
Clearance time (s)	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0
Lane Gap (vph)	3701	1162	94
Vs Ratio (Cap)	0.28	0.01	0.01
Vs Ratio Perm	0.02		
Vc Ratio	0.88	0.03	0.33
Uniform Delay d1	3.4	2.5	3.12
Progression radio	1.00	1.00	1.00
Incremental delay d2	0.1	0.0	0.21
Delay (s)	3.4	2.5	3.33
Level of Service	A	A	C
Approach Delay (s)	3.4	2.5	2.5
Approach LOS	A	A	A

Existing + Construction (Phase 1-3) AM
12/9/2011

1-3) AN
12/9/2014

HCM Signalized Intersection Capacity Analysis
5. Del Mar Heights Road & El Camino Real
12/29/2011

HCM Signalized Intersection Capacity Analysis (Phase 1-3) AM
12/29/2011
1: Del Mar Heights Road & I-15 SB Ramps

Movement	EBR	WBBL	WBTR	NBL	NBR	SBBL	SBTR
Lane Configurations							
Volume (vph)	217	874	212	1347	92	224	99
Ideal Flow (vphp)	1800	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	0.97
Flt. Protected	1.00	0.97	1.00	0.99	1.00	1.00	0.95
Flt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Said. Flow (vph)	3433	4836	3433	5037	3433	5065	3433
Flt. Permitted	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	4836	3433	5037	3433	5065	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flw (vph)	241	971	236	1497	102	249	110
RTO/R Reduction (vph)	0	48	0	0	10	0	69
Lane Group Flow (vph)	241	1159	0	209	1588	0	249
Turn Type	Prot						
Protected Phases	27	4	27	8	5	2	1
Permitted Phases	27	4	27	8	5	2	1
Adjusted Green, G(s)	9.7	24.0	9.6	23.9	7.0	12.5	9.3
Effective Green, g(s)	9.7	24.0	9.6	23.9	7.0	12.5	9.3
Adjusted g/C Ratio	0.14	0.34	0.13	0.33	0.10	0.18	0.13
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	466	1659	462	1886	337	890	277
vs Ratio Prot	0.07	0.23	0.06	0.32	0.07	0.02	0.01
vs Ratio Perm	0.07	0.23	0.06	0.32	0.07	0.02	0.01
vi Ratio	0.52	0.70	0.46	0.94	0.74	0.12	0.05
Uniform Delay, d1	20.7	20.6	28.5	29.1	31.3	24.8	26.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.0	1.3	0.7	1.1	8.2	0.1	0.6
Delay (s)	29.6	21.9	29.2	34.2	39.5	21.9	29.1
Level of Service	C	C	C	D	C	C	C
Approach Delay (s)	23.2	23.2	23.2	23.2	33.6	23.2	23.2
Approach LOS	C	C	C	C	C	C	C

Movement	EBR	WBBL	WBTR	NBL	NBR	SBBL	SBTR
Lane Configurations							
Volume (vph)	217	874	212	1347	92	224	99
Ideal Flow (vphp)	1800	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91	0.97
Flt. Protected	1.00	0.97	1.00	0.99	1.00	1.00	0.95
Flt. Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Said. Flow (vph)	3433	4836	3433	5037	3433	5065	3433
Flt. Permitted	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	4836	3433	5037	3433	5065	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flw (vph)	241	971	236	1497	102	249	110
RTO/R Reduction (vph)	0	48	0	0	10	0	69
Lane Group Flow (vph)	241	1159	0	209	1588	0	249
Turn Type	Prot						
Projected Phases	27	4	27	8	5	2	1
Permitted Phases	27	4	27	8	5	2	1
Adjusted Green, G(s)	9.7	24.0	9.6	23.9	7.0	12.5	9.3
Effective Green, g(s)	9.7	24.0	9.6	23.9	7.0	12.5	9.3
Adjusted g/C Ratio	0.14	0.34	0.13	0.33	0.10	0.18	0.13
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	466	1659	462	1886	337	890	277
vs Ratio Prot	0.07	0.23	0.06	0.32	0.07	0.02	0.01
vs Ratio Perm	0.07	0.23	0.06	0.32	0.07	0.02	0.01
vi Ratio	0.52	0.70	0.46	0.94	0.74	0.12	0.05
Uniform Delay, d1	20.7	20.6	28.5	29.1	31.3	24.8	26.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.0	1.3	0.7	1.1	8.2	0.1	0.6
Delay (s)	29.6	21.9	29.2	34.2	39.5	21.9	29.1
Level of Service	C	C	C	D	C	C	C
Approach Delay (s)	23.2	23.2	23.2	23.2	33.6	23.2	23.2
Approach LOS	C	C	C	C	C	C	C

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HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.

Existing + Construction (Phase 1-3) PM
12/9/2011

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Movement	TEB	EBR	WB	EWB	NBR	SWB
Lane Configurations	444	444	444	444	444	444
Volume (vph)	2145	32	21	1189	44	29
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	1.00	0.91	1.00	0.91
Flt	1.00	1.00	1.00	0.85	1.00	0.91
Flt Protected	1.00	1.00	1.00	0.95	1.00	0.95
Satl. Flow (prot)	5095	1563	3433	5085	1770	1583
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satl. Flow (perm)	5095	1563	3433	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Ajst. Flow (vph)	2318	23	1321	49	32	23
RTO/R Reduction (vph)	0	11	0	0	30	0
Lane Group Flow (vph)	2383	25	1324	49	32	23
Turn Type	Perm	Prot	Perm	Prot	Perm	Prot
Permitted Phases	4	2	2	4	2	2
Permitted Phases	4	2	2	4	2	2
Actuated Green, G (s)	365	0.6	43.1	43	43	43
Effective Green, g (s)	38.5	0.6	43.1	43	43	43
Actuated g/C Ratio	0.68	0.69	0.01	0.78	0.08	0.08
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3834	1100	37	3986	137	123
Vs Ratio Prot	0.47	0.01	0.03	0.26	0.03	0.02
Vs Ratio Perm	0.02	0.62	0.33	0.36	0.02	0.01
Vs Ratio	0.67	0.02	0.62	0.67	0.02	0.01
Uniform Delay, d1	4.9	2.6	27.3	1.8	24.2	23.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.0	28.2	0.1	1.6	0.1
Delay (s)	64	2.6	55.6	1.9	26.8	29.7
Level of Service	A	A	E	A	C	C
Approach Delay (s)	53	2.4	25.0	2.0	24.5	29.4
Approach LOS	A	A	C	A	D	C
Intersection Summary						
HCM Average Control Delay	4.9					
HCM Volume to Capacity ratio	0.65					
Actuated Cycle Length (s)	12.0					
Intersection Capacity Utilization	55.4					
Analysis Period (min)	15					
Critical Lane Group						

HCM Level of Service

C

HCM Average Control Delay

28.2

HCM Volume to Capacity ratio

0.74

Actuated Cycle Length (s)

12.0

Sum of lost time (s)

76.6

ICU Level of Service

65.9%

Analysis Period (min)

15

Critical Lane Group

Movement	TEB	EBR	WB	EWB	NBR	SWB
Lane Configurations	444	444	444	444	444	444
Volume (vph)	2145	32	21	1189	44	29
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	1.00	0.91	1.00	0.91
Flt	1.00	1.00	1.00	0.85	1.00	0.91
Flt Protected	1.00	1.00	1.00	0.95	1.00	0.95
Satl. Flow (prot)	5095	1563	3433	5085	1770	1583
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satl. Flow (perm)	5095	1563	3433	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Ajst. Flow (vph)	2318	23	1321	49	32	23
RTO/R Reduction (vph)	0	11	0	0	30	0
Lane Group Flow (vph)	2383	25	1324	49	32	23
Turn Type	Perm	Prot	Perm	Prot	Perm	Prot
Permitted Phases	4	2	2	4	2	2
Permitted Phases	4	2	2	4	2	2
Actuated Green, G (s)	365	0.6	43.1	43	43	43
Effective Green, g (s)	38.5	0.6	43.1	43	43	43
Actuated g/C Ratio	0.68	0.69	0.01	0.78	0.08	0.08
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3834	1100	37	3986	137	123
Vs Ratio Prot	0.47	0.01	0.03	0.26	0.03	0.02
Vs Ratio Perm	0.02	0.62	0.33	0.36	0.02	0.01
Vs Ratio	0.67	0.02	0.62	0.67	0.02	0.01
Uniform Delay, d1	4.9	2.6	27.3	1.8	24.2	23.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.0	28.2	0.1	1.6	0.1
Delay (s)	64	2.6	55.6	1.9	26.8	29.7
Level of Service	A	A	E	A	C	C
Approach Delay (s)	53	2.4	25.0	2.0	24.5	29.4
Approach LOS	A	A	C	A	D	C
Intersection Summary						
HCM Average Control Delay	4.9					
HCM Volume to Capacity ratio	0.65					
Actuated Cycle Length (s)	12.0					
Intersection Capacity Utilization	55.4					
Analysis Period (min)	15					
Critical Lane Group						

HCM Level of Service

C

HCM Average Control Delay

28.2

HCM Volume to Capacity ratio

0.74

Actuated Cycle Length (s)

12.0

Sum of lost time (s)

76.6

ICU Level of Service

65.9%

Analysis Period (min)

15

Critical Lane Group

HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Rd. & I-15 SB Ramps

HCM Signalized Intersection Capacity Analysis
Near Term + Construction (Phase 1-3) AM
3/5/2012

Movement	EBI	WB	WB	SEI	SEI	SR	SR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	0	1028	0	947	336	0	0
Ideal Flow (mph)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.97	0.91	0.97	0.95	0.95
Frt	1.00	1.00	0.99	0.85	1.00	1.00	0.85
Frt Protected	1.00	1.00	0.95	1.00	1.00	1.00	0.95
Satd. Flow (prot)	3639	3539	3430	1441	3433	3539	1441
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3639	3539	3430	1441	3433	3539	1441
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	770	1142	0	1052	373	0
RTOR Reduction (vph)	0	0	0	3	40	0	0
Lane Group Flow (vph)	0	770	1142	0	1086	256	0
Turn Type	Perm						
Protected Phases	26	62	4	4	4	6	8
Permitted Phases							
Actuated Green, G (s)	46.1	46.1	29.6	29.6	67.0	53.0	45.0
Effective Green, g (s)	46.1	46.1	29.6	29.6	67.0	53.0	45.0
Actuated g/C Ratio	0.53	0.53	0.34	0.34	0.56	0.44	0.44
Clearance Time (s)			5.6	5.6	4.0	4.0	4.0
Vehicle Extension (s)			3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1862	1862	1159	487	286	1976	2246
v/s Ratio Prot	0.22	0.32	0.32	0.32	0.07	0.42	0.33
v/s Ratio Perm							
v/s Ratio Perm	0.41	0.61	0.94	0.61	0.75	0.74	0.61
Uniform Delay, d1	12.6	14.5	28.1	24.2	54.5	20.1	31.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.6	13.8	2.1	28.3	2.6	2.2
Delay (s)	12.7	15.1	41.9	26.3	82.8	22.7	30.0
Level of Service	B	B	D	C	F	C	E
Approach Delay (s)	12.7	15.1	38.2	38.2	31.6	37.7	0.0
Approach LOS	B	B	D	D	C	D	A

Intersection Summary	C	C	C	C	C	C	C
HCM Average Control Delay	24.4	HCM Level of Service	C	C	C	C	C
HCM Volume to Capacity ratio	0.74						
Actuated Cycle Length (s)	87.6	Sum of lost time (s)	11.9				
Intersection Capacity Utilization	66.3%	ICU Level of Service	C	C	C	C	C
Analysis Period (min)	15						
c Critical Lane Group							

HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps

Near Term + Construction (Phase 1-3) AM
3/5/2012

Movement	EBI	EBI	WB	WB	SEI	SEI	SR	SR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	0	1028	0	947	336	0	0	0
Ideal Flow (mph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.97	0.91	0.97	0.95	0.95	0.95
Frt	1.00	1.00	0.99	0.85	1.00	1.00	0.85	0.85
Frt Protected	1.00	1.00	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	3639	3539	3430	1441	3433	3539	1441	1441
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	3639	3539	3430	1441	3433	3539	1441	1441
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	770	1142	0	1052	373	0	0
RTOR Reduction (vph)	0	0	0	3	40	0	0	0
Lane Group Flow (vph)	0	770	1142	0	1086	256	0	0
Turn Type	Perm							
Protected Phases	26	62	4	4	4	6	8	8
Permitted Phases								
Actuated Green, G (s)	46.1	46.1	29.6	29.6	67.0	53.0	45.0	45.0
Effective Green, g (s)	46.1	46.1	29.6	29.6	67.0	53.0	45.0	45.0
Actuated g/C Ratio	0.53	0.53	0.34	0.34	0.56	0.44	0.38	0.38
Clearance Time (s)			5.6	5.6	4.0	4.0	4.0	4.0
Vehicle Extension (s)			3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1862	1862	1159	487	286	1976	2246	2246
v/s Ratio Prot	0.22	0.32	0.32	0.32	0.07	0.42	0.33	0.33
v/s Ratio Perm								
v/s Ratio Perm	0.41	0.61	0.94	0.61	0.75	0.74	0.61	0.61
Uniform Delay, d1	12.6	14.5	28.1	24.2	54.5	20.1	31.4	31.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.6	13.8	2.1	28.3	2.6	2.2	2.2
Delay (s)	12.7	15.1	41.9	26.3	82.8	22.7	30.0	30.0
Level of Service	B	B	D	C	F	C	E	E
Approach Delay (s)	12.7	15.1	38.2	38.2	31.6	37.7	0.0	0.0
Approach LOS	B	B	D	D	C	D	C	C

Movement	EBI	EBI	WB	WB	SEI	SEI	SR	SR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	0	1028	0	947	336	0	0	0
Ideal Flow (mph)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3	5.6	5.6	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.97	0.91	0.97	0.95	0.95	0.95
Frt	1.00	1.00	0.99	0.85	1.00	1.00	0.88	0.85
Frt Protected	1.00	1.00	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	3639	3539	3430	1441	3433	3539	1441	1441
Frt Permitted	1.00	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	3639	3539	3430	1441	3433	3539	1441	1441
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	770	1142	0	1052	373	0	0
RTOR Reduction (vph)	0	0	0	3	40	0	0	0
Lane Group Flow (vph)	0	770	1142	0	1086	256	0	0
Turn Type	Perm							
Protected Phases	26	62	4	4	4	6	8	8
Permitted Phases								
Actuated Green, G (s)	46.1	46.1	29.6	29.6	67.0	53.0	45.0	45.0
Effective Green, g (s)	46.1	46.1	29.6	29.6	67.0	53.0	45.0	45.0
Actuated g/C Ratio	0.53	0.53	0.34	0.34	0.56	0.44	0.38	0.38
Clearance Time (s)			5.6	5.6	4.0	4.0	4.0	4.0
Vehicle Extension (s)			3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1862	1862	1159	487	286	1976	2246	2246
v/s Ratio Prot	0.22	0.32	0.32	0.32	0.07	0.42	0.33	0.33
v/s Ratio Perm								
v/s Ratio Perm	0.41	0.61	0.94	0.61	0.75	0.74	0.61	0.61
Uniform Delay, d1	12.6	14.5	28.1	24.2	54.5	20.1	31.4	31.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.6	13.8	2.1	28.3	2.6	2.2	2.2
Delay (s)	12.7	15.1	41.9	26.3	82.8	22.7	30.0	30.0
Level of Service	B	B	D	C	F	C	E	E
Approach Delay (s)	12.7	15.1	38.2	38.2	31.6	37.7	0.0	0.0
Approach LOS	B	B	D	D	C	D	C	C

Intersection Summary

HCM Average Control Delay

HCM Volume to Capacity ratio

Actualized Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

c Critical Lane Group

D HCM Level of Service

E Sum of lost time (s)

F ICU Level of Service

G 15

H Baseline

I Synchro 7 - Report

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HCM Signalized Intersection Capacity Analysis
3: Del Mar Heights Road & High Bluff Drive

Near Term + Construction (Phase 1-3) AM
3/5/2012
4: Del Mar Heights Road & First Ave.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBL	SBR
Lane Configurations												
Volumes (vph)	111	1448	694	95	1004	61	201	10	13	81	59	312
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.87	0.95	1.00	1.00	1.00	1.00	1.00
Flt	1.00	1.00	0.85	1.00	1.00	1.00	0.92	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00
Sad. Flow (vph)	1770	5085	1583	1770	5062	3433	3242	1770	1863	1683	1683	1683
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sad. Flow (perm)	1770	5085	1583	1770	5062	3433	3242	1770	1863	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1609	771	106	2116	68	223	11	14	90	66	347
R/TOR Reduction (vph)	0	0	0	0	4	0	0	12	0	0	134	0
Lane Group Flow (vph)	123	1609	396	106	2180	0	223	13	0	90	66	213
Turn Type	Prot											
Protected Phases	7	4	3	8	5	2	1	6	6	6	6	6
Permitted Phases												
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2	17.4	17.4	17.4	17.4
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2	17.4	17.4	17.4	17.4
Actuated g/C Ratio	0.08	0.47	0.47	0.08	0.45	0.08	0.13	0.15	0.20	0.20	0.20	0.20
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	160	2376	740	134	2290	272	411	264	367	312	312	312
v/s Ratio Prot	0.07	0.32	0.25	0.06	0.03	0.06	0.01	0.05	0.04	0.13	0.13	0.13
v/s Ratio Perm												
v/s Ratio	0.77	0.68	0.54	0.79	0.95	0.82	0.03	0.34	0.18	0.68	0.68	0.68
Uniform Delay, d1	39.3	18.4	16.7	40.2	23.3	40.1	33.8	33.7	29.6	32.9	32.9	32.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.8	0.7	26.4	9.9	17.3	0.0	0.8	0.2	6.0	2.7	0.1
Delay (s)	58.9	19.1	17.5	66.6	33.1	57.4	35.9	34.5	29.8	39.0	38.8	35.2
Level of Service	E	B	B	C	C	E	C	C	C	D	A	C
Approach Delay (s)	20.6	34.7	32.6	37.0	55.0	37.0	37.0	37.0	37.0	32.6	32.6	32.6
Approach LOS	C	C	C	D	D	D	D	D	D	A	A	C

Intersection Summary												
HCM Average Control Delay	29.4	HCM Level of Service	C									
HCM Volume to Capacity ratio	0.95											
Actuated Cycle Length (s)	16.0	Sum of lost time (s)	88.4									
Intersection Capacity Utilization	73.2%	ICU Level of Service	D									
Analysis Period (min)	15											
c Critical Lane Group												

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBL	SBR
Lane Configurations												
Volume (vph)	111	1448	694	95	1004	61	201	10	13	81	59	312
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.87	0.95	1.00	1.00	1.00	1.00	1.00
Flt	1.00	1.00	0.85	1.00	1.00	1.00	0.92	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00
Sad. Flow (vph)	1770	5085	1583	1770	5062	3433	3242	1770	1863	1683	1683	1683
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sad. Flow (perm)	1770	5085	1583	1770	5062	3433	3242	1770	1863	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1609	771	106	2116	68	223	11	14	90	66	347
R/TOR Reduction (vph)	0	0	0	0	4	0	0	12	0	0	134	0
Lane Group Flow (vph)	123	1609	396	106	2180	0	223	13	0	90	66	213
Turn Type	Prot											
Protected Phases	7	4	3	8	5	2	1	6	6	6	6	6
Permitted Phases												
Actuated Green, G (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2	17.4	17.4	17.4	17.4
Effective Green, g (s)	8.0	41.3	41.3	6.7	40.0	7.0	11.2	13.2	17.4	17.4	17.4	17.4
Actuated g/C Ratio	0.08	0.47	0.47	0.08	0.45	0.08	0.13	0.15	0.20	0.20	0.20	0.20
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	160	2376	740	134	2290	272	411	264	367	312	312	312
v/s Ratio Prot	0.07	0.32	0.25	0.06	0.03	0.06	0.01	0.05	0.04	0.13	0.13	0.13
v/s Ratio Perm												
v/s Ratio	0.77	0.68	0.54	0.79	0.95	0.82	0.03	0.34	0.18	0.68	0.68	0.68
Uniform Delay, d1	39.3	18.4	16.7	40.2	23.3	40.1	33.8	33.7	29.6	32.9	32.9	32.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.6	0.8	0.7	26.4	9.9	17.3	0.0	0.8	0.2	6.0	2.7	0.1
Delay (s)	58.9	19.1	17.5	66.6	33.1	57.4	35.9	34.5	29.8	39.0	38.8	35.2
Level of Service	E	B	B	C	C	E	C	C	C	D	A	C
Approach Delay (s)	20.6	34.7	32.6	37.0	55.0	37.0	37.0	37.0	37.0	32.6	32.6	32.6
Approach LOS	C	C	C	D	D	D	D	D	D	A	A	C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBT	SBL	SBR
Lane Configurations												
Volume (vph)	111	1448	694	95	1004	61	201	10	13	81	59	312
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	0.87	0.95	1.00	1.00	1.00	1.00	1.00
Flt	1.00	1.00	0.85	1.00	1.00	1.00	0.92	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00
Sad. Flow (vph)	1770	5085	1583	1770	5062	3433	3242	1770	1863	1683	1683	1683
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Sad. Flow (perm)	1770	5085	1583	1770	5062	3433	3242	1770	1863	1683	1683	1683
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	1609	771	106	2116	68	223	11	14	90	66	347
R/TOR Reduction (vph)	0	0	0	0	4	0	0	12	0	0	134	0
Lane Group Flow (vph)	123	1609	396	106	2180	0	223	13	0	90	66	213
Turn Type	Prot											
Protected Phases	7	4	3	8	5	2	1	6	6	6	6	6
Permitted Phases												
Actuated Green, G (s)	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9
Effective Green, g (s)	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9	50.9
Actuated g/C Ratio	0.74	0.74										

HCM Signalized Intersection Capacity Analysis
5: Del Mar Heights Road & El Camino Real

Near Term + Construction (Phase 1-3) AM
3/5/2012
HCM Signalized Intersection Capacity Analysis
1: Del Mar Heights Road & I-15 SB Ramps

Movement	EBL	EBR	WBL	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	223	900	405	206	1388	95	253	104
Ideal Flow (vphpl)	1800	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	0.97	0.91	0.97	0.91	1.00	0.97	0.91
Frt	1.00	0.95	1.00	0.99	1.00	1.00	0.85	1.00
Frt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Sdt. Flow (prot)	3433	4849	3433	5036	3433	5036	1683	3433
Frt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Sdt. Flow (perm)	3433	4849	3433	5036	3433	5036	1583	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	248	1000	450	229	1542	106	281	116
R/T/R Reduction (vph)	0	106	0	0	0	0	0	0
Lane Group Flow (vph)	248	1344	0	229	1638	0	281	116
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6
Permitted Phases								
Actuated Green, G (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Effective Green, g (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.13	0.23
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	325	1651	413	1844	325	976	304	450
v/s Ratio Prot	0.07	0.28	0.07	0.33	0.08	0.02	0.01	0.05
v/s Ratio Perm								
v/c Ratio	0.76	0.81	0.85	0.88	0.86	0.12	0.06	0.40
Uniform Delay, d1	32.7	22.3	30.7	22.0	33.0	24.7	24.4	25.6
Progression Factor	-1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	10.2	3.2	1.6	5.6	20.5	0.1	0.1	1.1
Delay (s)	42.9	25.5	32.3	27.7	53.6	24.8	24.5	26.7
Level of Service	D	C	C	C	D	C	C	C
Approach Delay (s)	28.0	28.2	28.4	28.4	41.4	28.2	27.3	27.3
Approach LOS	C	C	C	C	D	C	C	C

Intersection Summary	C
HCM Average Control Delay	22.5
HCM Volume to Capacity ratio	0.79
Actuated Cycle Length (s)	87.7
Intersection Capacity Utilization	71.1%
Analysis Period (min)	15
c Defacto Right Lane. Recode with 1 through lane as a right lane.	
c Critical Lane Group	

Movement	EEL	EEB	EEF	EEI	EEJ	EEB	EEF	EEI	EEJ	SBT
Lane Configurations	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Volume (vph)	223	900	405	206	1388	95	253	104	79	164
Ideal Flow (vphpl)	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	1.00	0.99	0.97	1.00	0.97	0.91	1.00	0.91
Frt	1.00	0.95	1.00	0.99	1.00	1.00	0.85	1.00	1.00	0.85
Frt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Sdt. Flow (prot)	3433	4849	3433	5036	3433	5036	1683	3433	4647	3433
Frt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Sdt. Flow (perm)	3433	4849	3433	5036	3433	5036	1583	3433	4647	3433
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	248	1000	450	229	1542	106	281	116	88	182
R/T/R Reduction (vph)	0	106	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	248	1344	0	229	1638	0	281	116	71	0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	1	6	2	4
Permitted Phases										
Actuated Green, G (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9	45.7	45.7
Effective Green, g (s)	7.0	25.2	8.9	27.1	7.0	14.2	9.7	16.9	45.7	45.7
Actuated g/C Ratio	0.09	0.34	0.12	0.37	0.09	0.19	0.13	0.23	0.52	0.52
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	325	1651	413	1844	325	976	304	450	1061	1844
v/s Ratio Prot	0.07	0.28	0.07	0.33	0.08	0.02	0.01	0.05	0.14	0.31
v/s Ratio Perm										
v/c Ratio	0.76	0.81	0.85	0.88	0.86	0.12	0.06	0.40	0.62	0.62
Uniform Delay, d1	32.7	22.3	30.7	22.0	33.0	24.7	24.4	25.6	14.1	16.2
Progression Factor	-1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	10.2	3.2	1.6	5.6	20.5	0.1	0.1	1.1	0.4	1.5
Delay (s)	42.9	25.5	32.3	27.7	53.6	24.8	24.5	26.7	14.5	17.6
Level of Service	D	C	C	C	D	C	C	C	B	B
Approach Delay (s)	28.0	28.2	28.4	28.4	41.4	28.2	27.3	27.3	14.5	17.6
Approach LOS	C	C	C	C	D	C	C	C	B	B

Lane Configurations	Volume (vph)	Ideal Flow (vphpl)	Total Lost time (s)	Lane Util. Factor	Frt	Frt Protected	Sdt. Flow (prot)	Frt Permitted	Sdt. Flow (perm)	Peak-hour factor, PHF	Adj. Flow (vph)	R/T/R Reduction (vph)	Lane Group Flow (vph)	Turn Type	Protected Phases	Permitted Phases	Actuated Green, G (s)	Effective Green, g (s)	Actuated g/C Ratio	Clearance Time (s)	Vehicle Extension (s)	Lane Grp Cap (vph)	v/s Ratio Prot	v/s Ratio Perm	v/c Ratio	Uniform Delay, d1	Progression Factor	Incremental Delay, d2	Delay (s)	Level of Service	Approach Delay (s)	Approach LOS	Intersection Summary	C
↑↑↑	0	0	0	1.00	1.00	1.00	0	0	0	0.90	0.90	0	0	Prot	7	4	7.0	7.0	0.09	1.00	1.00	325	0.07	0.07	0.76	32.7	-1.00	10.2	42.9	D	28.0	C	Intersection Summary	C
↑↑↑	915	1900	1900	1.00	1.00	1.00	915	1900	1900	0.90	0.90	0	0	Prot	7	4	7.0	7.0	0.09	1.00	1.00	325	0.07	0.07	0.76	32.7	-1.00	10.2	42.9	D	28.0	C	Intersection Summary	C
↑↑↑	915	1900	1900	1.00	1.00	1.00	915	1900	1900	0.90	0.90	0	0	Prot	7	4	7.0	7.0	0.09	1.00	1.00	325	0.07	0.07	0.76	32.7	-1.00	10.2	42.9	D	28.0	C	Intersection Summary	C
↑↑↑	915	1900	1900	1.00	1.00	1.00	915	1900	1900	0.90	0.90	0	0	Prot	7	4	7.0	7.0	0.09	1.00	1.00	325	0.07	0.07	0.76	32.7	-1.00	10.2	42.9	D	28.0	C	Intersection Summary	C

Baseline

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HCM Signalized Intersection Capacity Analysis
2: Del Mar Heights Road & I-15 NB Ramps

Near Term + Construction (Phase 1-3) PM
3: Del Mar Heights Road & High Bluff Drive

Movement	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations											
Volume (vph)	242	1533	0	0	1216	916	649	24	816	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	1.00	0.95	0.91	0.95	1.00	0.91	1.00	1.00
Filt	1.00	1.00	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00
Filt Protected	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00
Said. Flow (prot)	3433	3539	0	0	5085	1583	1881	1500	1504		
Filt Permitted	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	3539	0	0	5085	1583	1881	1500	1504		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1103	0	0	1351	1011	721	27	907	0	0
RTOR Reduction (vph)	0	0	0	0	0	509	0	6	6	0	0
Lane Group Flow (vph)	269	1703	0	0	1351	502	577	546	520	0	0
Turn Type	Prot				Prot	Spill			Prot		
Protected Phases	5	2			6	6	8	8	8		
Permitted Phases											
Actuated Green, G (s)	11.2	62.1	11.2	62.1	46.9	46.9	49.9	49.9	49.9		
Effective Green, g (s)	11.2	62.1	11.2	62.1	46.9	46.9	49.9	49.9	49.9		
Actuated g/C Ratio	0.09	0.52			0.39	0.39	0.42	0.42	0.42		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	320	1831	1987	619	699	624	625				
vs Ratio Prot	0.08	0.48			0.27	0.32	0.34	0.36	0.35		
vs Ratio Perm											
vc Ratio	0.84	0.93			0.68	0.81	0.83	0.88	0.83		
Uniform Delay, d1	53.5	26.9			30.3	32.6	31.2	32.2	31.3		
Progression Factor	1.00	1.00			1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	17.7	9.9			19	11.0	7.9	13.0	9.3		
Delay (s)	71.2	36.9			32.2	43.6	39.1	45.2	40.6		
Level of Service	E	D	C	D	D	D	D	D	D	D	D
Approach Delay (s)	41.6		37.1		41.6		41.6		A		
Approach LOS	D		D		D		D		D		D

Intersection Summary

HCM Average Control Delay	39.8	HCM Level of Service	D
HCM Volume to Capacity ratio	0.91		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	100.2%	ICU Level of Service	G
Analysis Period (min)	15	c Critical Lane Group	

Baseline

Synchro 7 - Report
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Near Term + Construction (Phase 1-3) PM
3: Del Mar Heights Road & High Bluff Drive

Movement	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations											
Volume (vph)	242	1533	0	0	1216	916	649	24	816	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	1.00	0.95	0.91	0.95	1.00	0.91	1.00	1.00
Filt	1.00	1.00	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00
Filt Protected	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00
Said. Flow (prot)	3433	3539	0	0	5085	1583	1881	1500	1504		
Filt Permitted	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	3539	0	0	5085	1583	1881	1500	1504		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1103	0	0	1351	1011	721	27	907	0	0
RTOR Reduction (vph)	0	0	0	0	0	509	0	6	6	0	0
Lane Group Flow (vph)	269	1703	0	0	1351	502	577	546	520	0	0
Turn Type	Prot				Prot	Spill			Prot		
Protected Phases	5	2			6	6	8	8	8		
Permitted Phases											
Actuated Green, G (s)	11.2	62.1	11.2	62.1	46.9	46.9	49.9	49.9	49.9		
Effective Green, g (s)	11.2	62.1	11.2	62.1	46.9	46.9	49.9	49.9	49.9		
Actuated g/C Ratio	0.09	0.52			0.39	0.39	0.42	0.42	0.42		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	320	1831	1987	619	699	624	625				
vs Ratio Prot	0.08	0.48			0.27	0.32	0.34	0.36	0.35		
vs Ratio Perm											
vc Ratio	0.84	0.93			0.68	0.81	0.83	0.88	0.83		
Uniform Delay, d1	53.5	26.9			30.3	32.6	31.2	32.2	31.3		
Progression Factor	1.00	1.00			1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	17.7	9.9			19	11.0	7.9	13.0	9.3		
Delay (s)	71.2	36.9			32.2	43.6	39.1	45.2	40.6		
Level of Service	E	D	C	D	D	D	D	D	D	D	D
Approach Delay (s)	41.6		37.1		41.6		41.6		A		
Approach LOS	D		D		D		D		D		D

Movement	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations											
Volume (vph)	242	1533	0	0	1216	916	649	24	816	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	0.91	1.00	0.95	0.91	0.95	1.00	0.91	1.00	1.00
Filt	1.00	1.00	1.00	1.00	0.85	1.00	0.90	0.85	1.00	1.00	1.00
Filt Protected	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00
Said. Flow (prot)	3433	3539	0	0	5085	1583	1881	1500	1504		
Filt Permitted	0.95	1.00	1.00	1.00	0.95	0.99	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	3539	0	0	5085	1583	1881	1500	1504		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	269	1103	0	0	1351	1011	721	27	907	0	0
RTOR Reduction (vph)	0	0	0	0	0	509	0	6	6	0	0
Lane Group Flow (vph)	269	1703	0	0	1351	502	577	546	520	0	0
Turn Type	Prot				Prot	Spill			Prot		
Protected Phases	5	2			6	6	8	8	8		
Permitted Phases											
Actuated Green, G (s)	11.2	62.1	11.2	62.1	46.9	46.9	49.9	49.9	49.9		
Effective Green, g (s)	11.2	62.1	11.2	62.1	46.9	46.9	49.9	49.9	49.9		
Actuated g/C Ratio	0.09	0.52			0.39	0.39	0.42	0.42	0.42		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gap Cap (vph)	320	1831	1987	619	699	624	625				
vs Ratio Prot	0.08	0.48			0.27	0.32	0.34	0.36	0.35		
vs Ratio Perm											
vc Ratio	0.84	0.93			0.68	0.81	0.83	0.88	0.83		
Uniform Delay, d1	53.5	26.9			30.3	32.6	31.2	32.2	31.3		
Progression Factor	1.00	1.00			1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	17.7	9.9			19	11.0	7.9	13.0	9.3		
Delay (s)	71.2	36.9			32.2	43.6	39.1	45.2	40.6		
Level of Service	E	D	C	D	D	D	D	D	D	D	D
Approach Delay (s)	41.6		37.1		41.6		41.6		A		
Approach LOS	D		D		D		D		D		D

Movement	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR

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HCM Signalized Intersection Capacity Analysis
4: Del Mar Heights Road & First Ave.

HCM Signalized Intersection Capacity Analysis
Near Term + Construction (Phase 1-3) PM
3/5/2012
5: Del Mar Heights Road & El Camino Real

Movement	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations						
Volume (vph)	2301	33	22	1588	47	32
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.91	1.00	0.97	1.00	1.00	0.97
Fit	1.00	1.00	0.85	1.00	1.00	0.85
Fit Protected	1.00	0.95	1.00	0.95	1.00	0.95
Said. Flow (prot)	5085	1583	3433	5085	1770	1583
Fit Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	5085	1583	3433	5085	1770	1583
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	2557	37	24	1764	52	36
RTO/R Reduction (vph)	0	11	0	0	33	0
Lane Group Flow (vph)	2557	26	24	1764	52	3
Turn Type	Perm	Prot	Perm	Prot	Perm	Prot
Protected Phases	4	3	8	2	2	2
Permitted Phases	4	4	2	2	2	2
Actuated Green, G (s)	39.6	39.6	0.6	44.2	4.3	4.3
Effective Green, g (s)	39.6	39.6	0.6	44.2	4.3	4.3
Actuated g/C Ratio	0.70	0.70	0.01	0.78	0.08	0.08
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	3654	1110	36	3978	135	120
vs Ratio Prot	0.50	0.02	0.01	0.35	0.03	0.00
vs Ratio Perm	0.50	0.02	0.01	0.35	0.03	0.00
vc Ratio	0.72	0.02	0.67	0.44	0.39	0.02
Uniform Delay, d1	5.1	2.6	27.9	2.0	24.8	24.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.7	0.0	0.38	0.1	1.8	0.1
Delay (s)	5.8	2.6	65.8	2.1	26.7	24.2
Level of Service	A	A	E	A	C	C
Approach Delay (s)	5.7	3.0	25.7	3.0	25.7	3.0
Approach LOS	A	A	C	A	C	C
Intersection Summary						
HCM Average Control Delay	5.0	HCM Level of Service		A	D	
HCM Volume to Capacity ratio	0.70	HCM Level of Service		A	D	
Actuated Cycle Length (s)	56.5	Sum of lost time (s)		120	16.0	
Intersection Capacity Utilization	54.5%	ICU Level of Service		A	D	
Analysis Period (min)	15	Analysis Period (min)		15	15	
c Critical Lane Group		c Critical Lane Group				

Movement	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations						
Volume (vph)	1400	419	181	769	108	458
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.91	0.97	0.91	0.97	0.91
Fit	1.00	1.00	1.00	1.00	1.00	1.00
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (prot)	3433	4909	3433	4909	3433	4909
Fit Permitted	0.95	1.00	0.95	1.00	0.95	1.00
Said. Flow (perm)	3433	4909	3433	4909	3433	4909
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	507	1556	466	120	854	201
RTO/R Reduction (vph)	0	55	0	0	41	0
Lane Group Flow (vph)	507	1957	0	120	1014	0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2
Permitted Phases	7	4	3	8	5	2
Actuated Green, G (s)	16.5	40.4	4.6	28.5	10.1	15.4
Effective Green, g (s)	16.5	40.4	4.6	28.5	10.1	15.4
Actuated g/C Ratio	0.20	0.49	0.06	0.34	0.12	0.19
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	692	2387	190	1694	417	942
vs Ratio Prot	0.15	0.040	0.03	0.21	0.15	0.05
vs Ratio Perm	0.15	0.040	0.03	0.21	0.15	0.05
vc Ratio	0.74	0.82	0.63	0.60	1.22	0.50
Uniform Delay, d1	31.3	18.3	38.4	22.6	36.5	30.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	4.4	2.4	6.7	0.6	119.2	0.4
Delay (s)	35.7	20.1	46.1	23.1	155.7	33.3
Level of Service	D	C	D	C	F	C
Approach Delay (s)	23.7	25.4	25.4	25.4	81.7	C
Approach LOS	C	C	C	C	F	C
Intersection Summary						
HCM Average Control Delay	38.4	HCM Level of Service		D		
HCM Volume to Capacity ratio	0.84	HCM Level of Service		A		
Actuated Cycle Length (s)	83.1	Sum of lost time (s)		120	16.0	
Intersection Capacity Utilization	73.6%	ICU Level of Service		A	D	
Analysis Period (min)	15	Analysis Period (min)		15	15	
c Critical Lane Group		c Critical Lane Group				